



University of Pittsburgh

Unbonded Concrete Overlays of Existing Concrete Pavements (UBOLs): Task 3 Structural Model Development

TAP Meeting
February 22, 2016

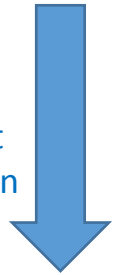
Julie Vandenbossche, Ph.D., P.E.
Associate Professor



Task 2: Consolidation

Permanent deformation in HMA

Increasing
permanent
deformation

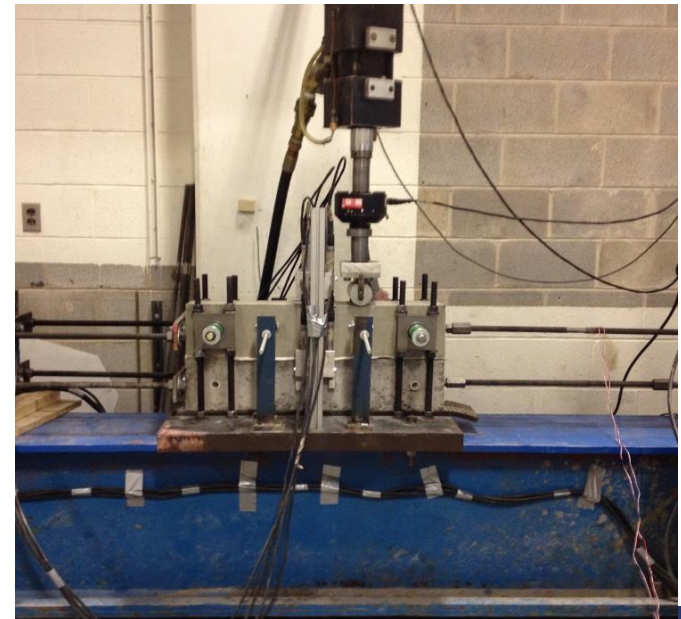


MI open graded
MI dense graded
MN dense graded
MN open graded

Fabric not influenced



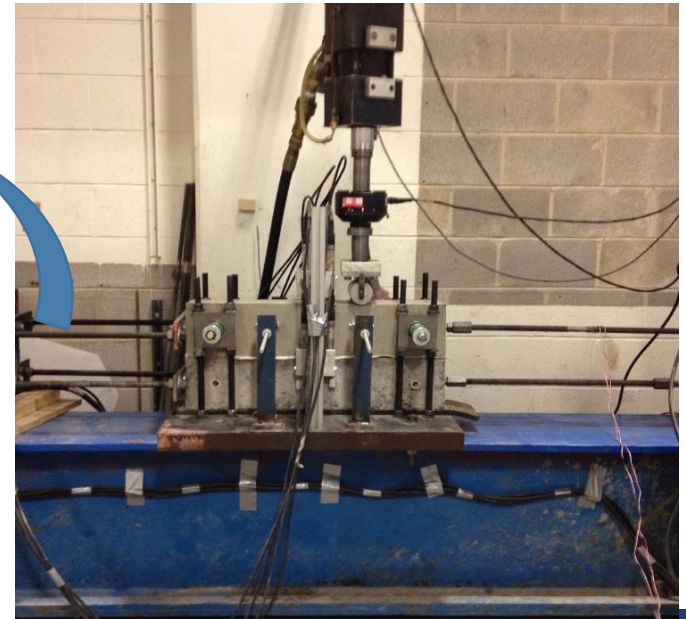
Longitudinal outer wheel path crack in LTPP section



Task 2: Fatigue

Fatigue in interlayer

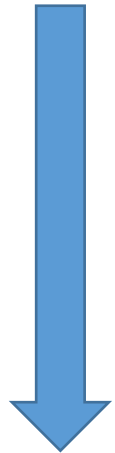
- MN open graded
- None exhibited in other interlayers



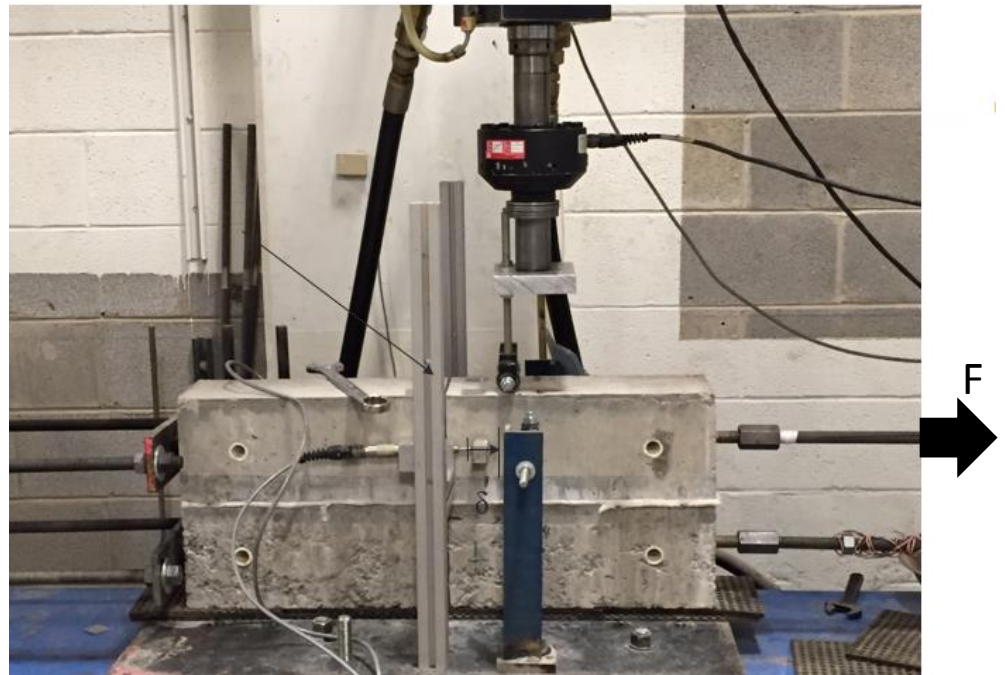
Task 2: Friction

Frictional restraint

Decreasing
frictional
restraint



MI open graded
MI dense graded
MN dense graded
MN open graded
10 oz fabric glued
10 oz fabric pinned
15 oz fabric glued
15 oz fabric pinned



Task 2: Friction

Too much vs too little

- All joints not working (large effective panel size)
- Result: Jt deterioration and/or fatigue cracking

Not deployed (Approx. 0.25-in wide)



Deployed (Approx. 0.5-in wide)

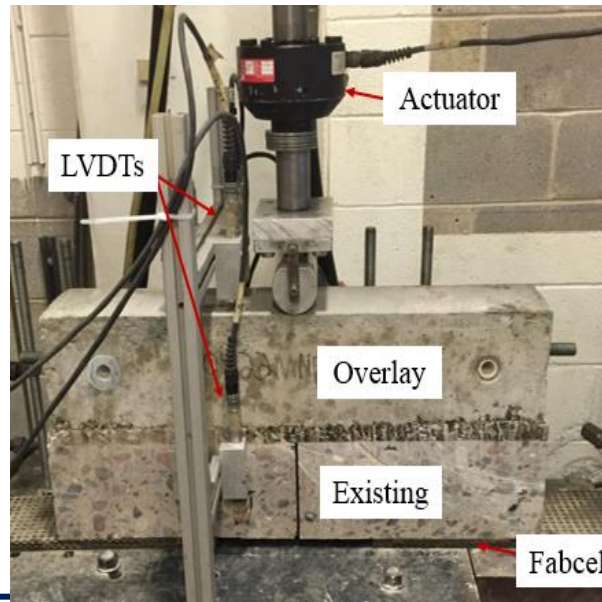


SR 50 UBOL in Bridgeville, PA

University of Pittsburgh Department of Civil and Environmental Engineering

Task 2: Reflective cracking

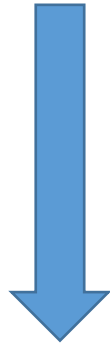
- Reflective cracking not generated when fully supported
- Fabric tends to increase resistance to reflective cracking when compared to HMA
- MI open graded appears to perform better than other HMA interlayers
 - Greater resistance to reflective cracking and less permanent deformation



Task 2: Direct tension

- Examine curling warping stresses
- Measure vertical deformations within interlayer and interface strength

Decreasing
resistance to
vertical uplift



MI dense graded
MN dense graded
MI open graded
MN open graded
10 oz fabric
15 oz fabric



Joint performance

- Erosion
- Consolidation
- Fatigue
- Faulting

Interlayer erosion

Contributing Factors

- Moisture
- Traffic
- Asphalt susceptibility
- Drainage

US-23 in MI



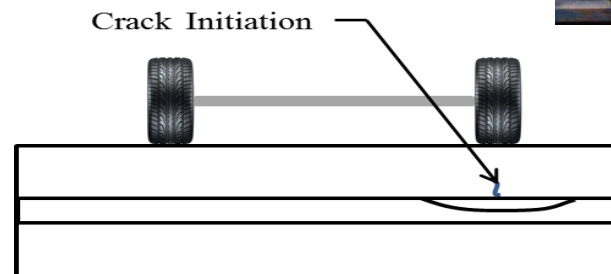
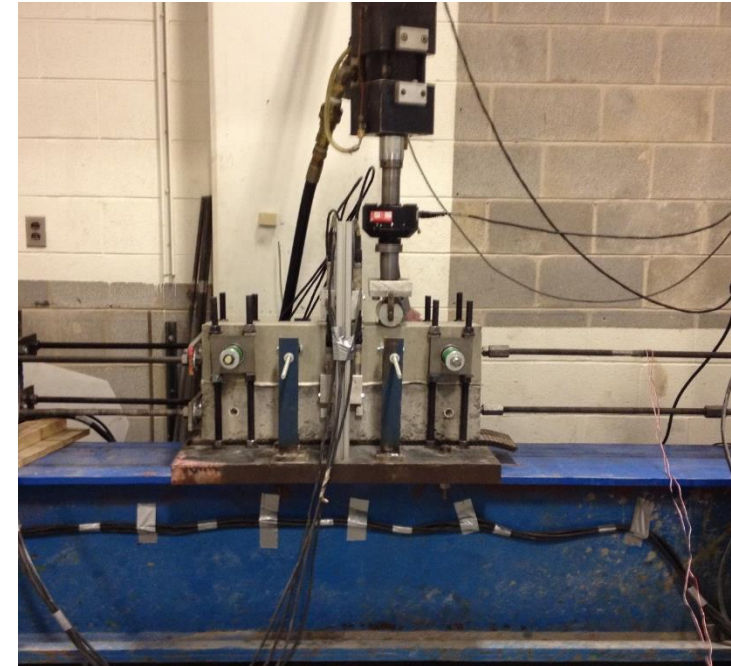
Photo courtesy of Andy Bennett

Interlayer erosion

- Erodibility factor
 - Interlayer drainability
 - Binder content
 - Film thickness
 - Permeability
 - Binder and aggregate type
- Predicted erosion depth
 - Critical response from FEM rapid solution
 - Erodibility factor

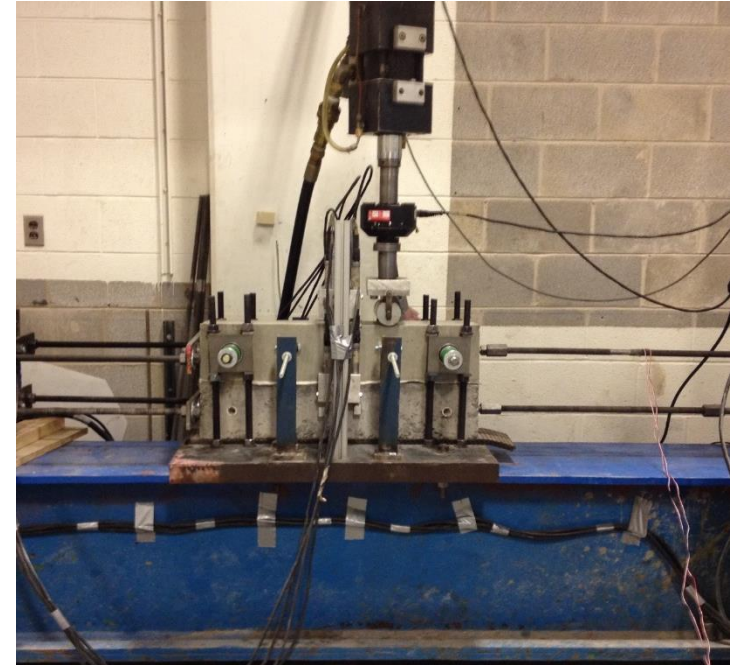
Consolidation

- Void created in interlayer
- Observed in lab testing
- Contributing factors
 - Traffic
 - Asphalt compressive strength
 - Vertical interlayer strain
 - LTE and Mag. of deflection



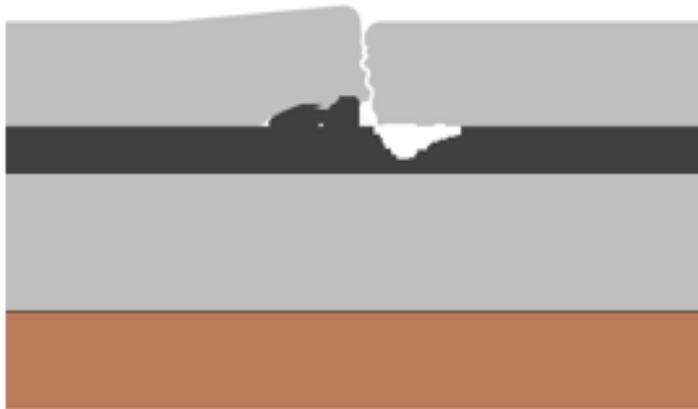
Interlayer fatigue

- Observed in lab testing
- Contributing factors
 - Traffic
 - Asphalt compressive strength
 - Vertical interlayer strain
 - LTE and mag. of deflection

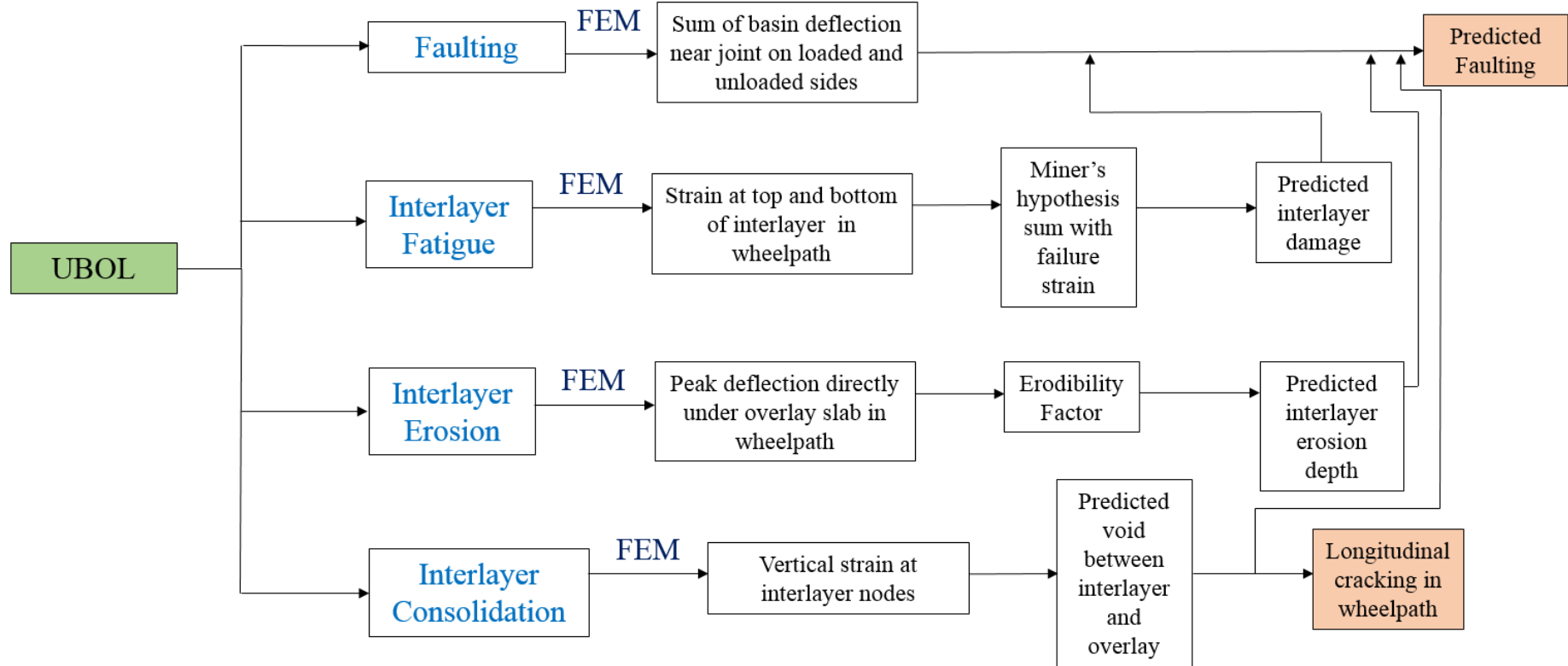


Faulting

- Occurs in HMA interlayer (not fabric)
- Affected by
 - Fatigue
 - Erosion
 - Consolidation



Faulting prediction framework

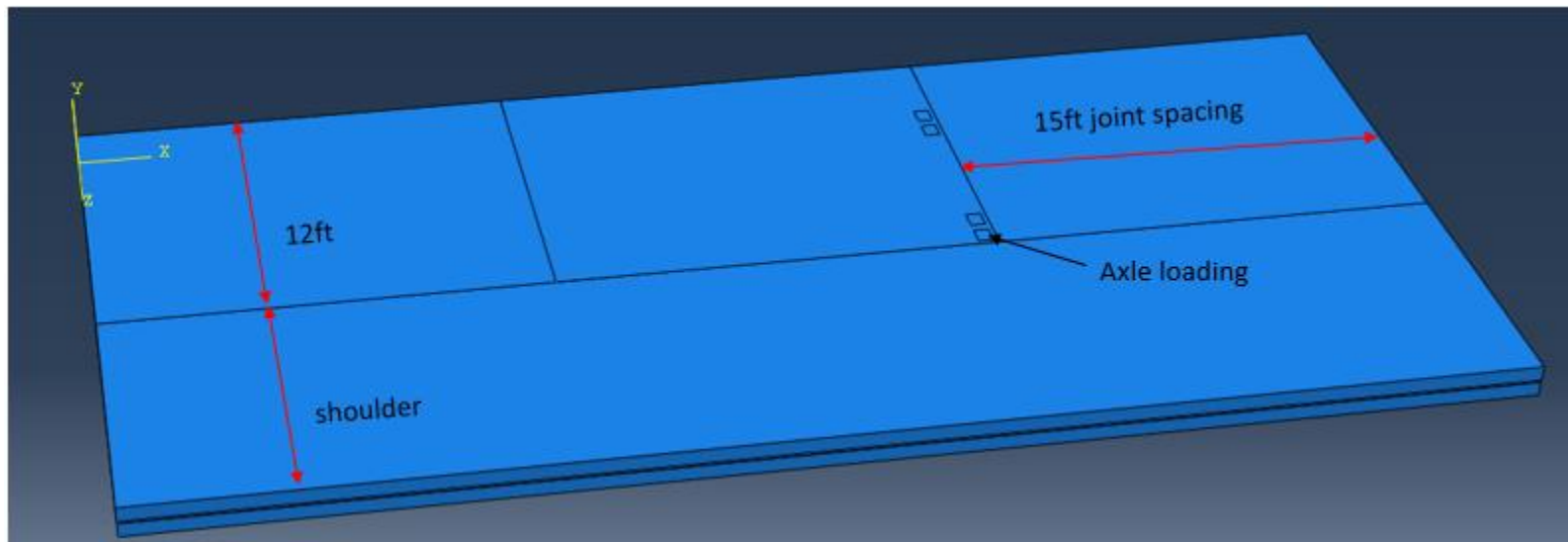


Neural Network development

- FEM results to develop rapid solutions
- NN trained to predict critical responses

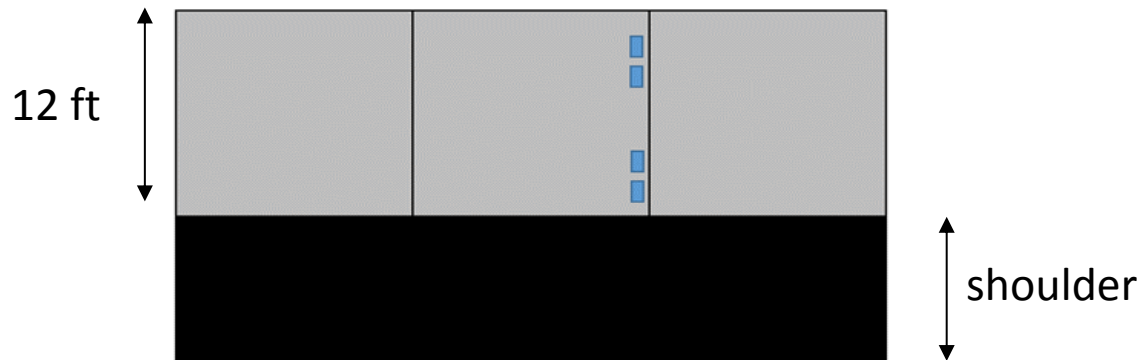
Joint faulting response

- 3D ABAQUS Model
- Model:
 - 3 overlay slabs
 - Shoulder
 - Asphalt Interlayer (no fabric)
 - Existing PCC

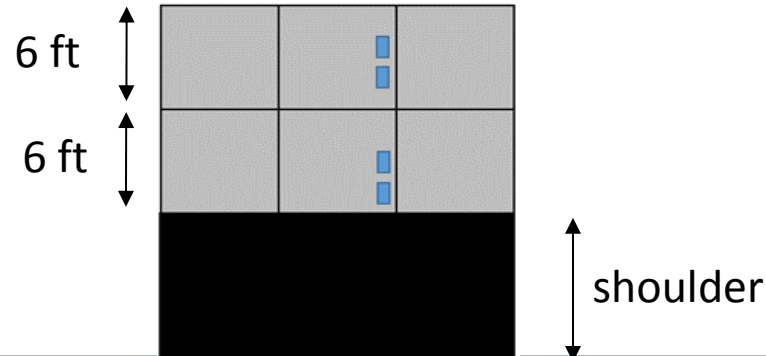


Slab sizes

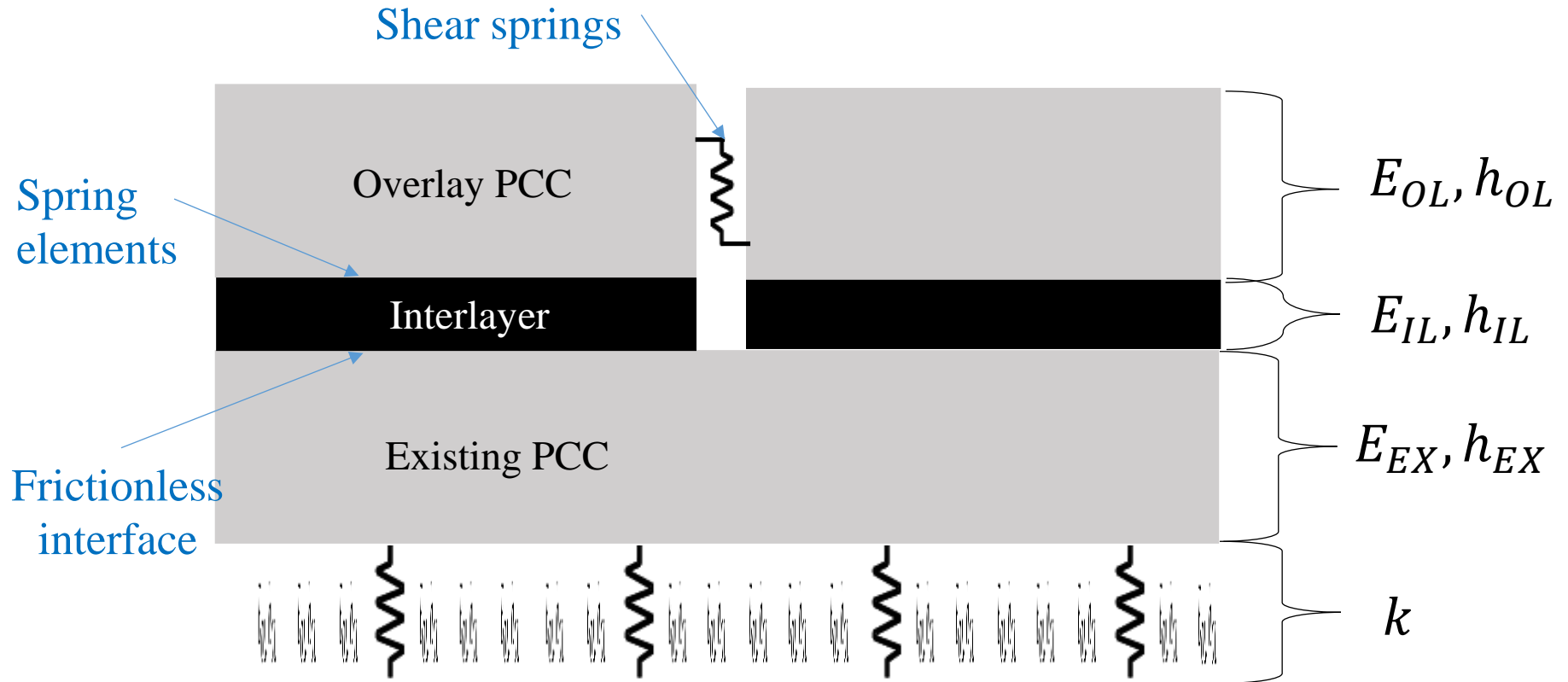
- 3 overlay slabs
- 12 ft lane (jt. spacing = 10, 15, 20 ft)



- 6 ft x 6 ft slabs

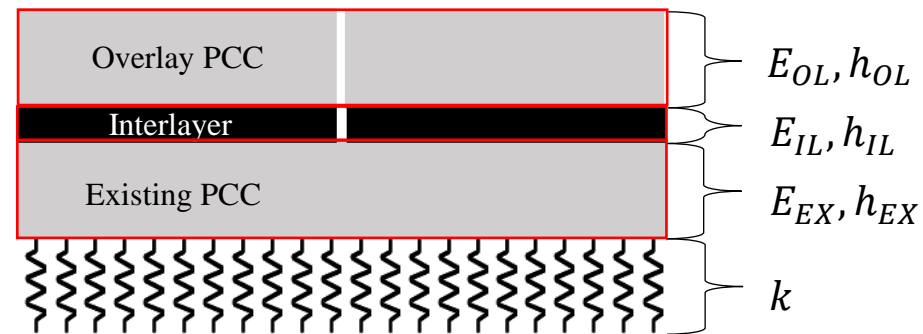


Modeling properties



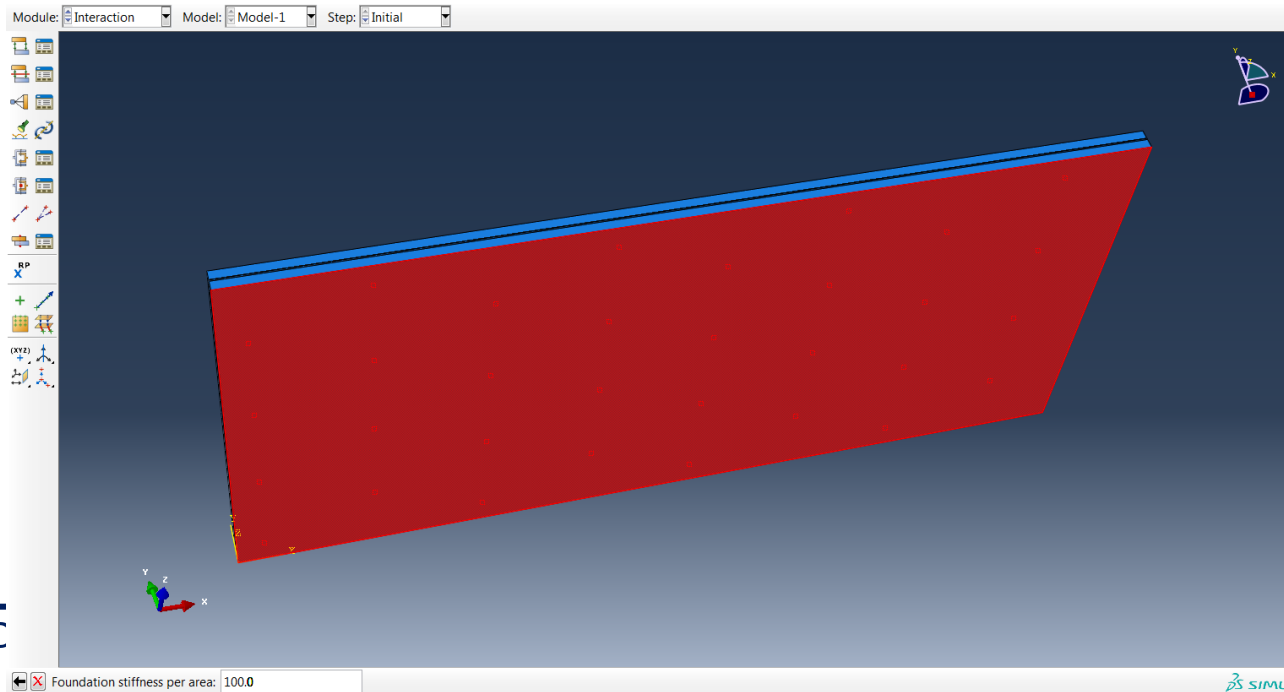
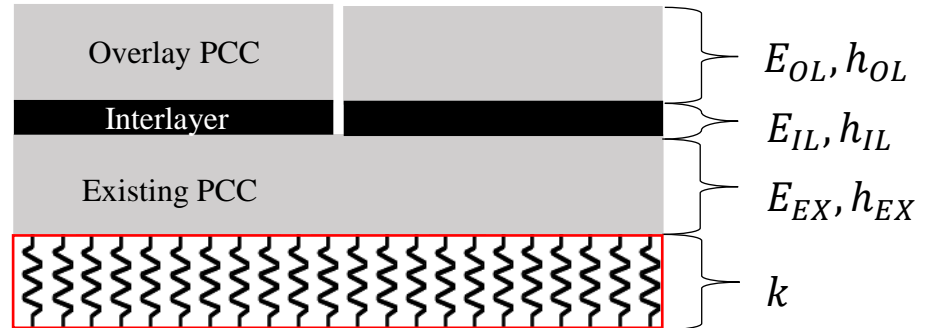
PCC and asphalt layers

- Elastic solids
 - E
 - $\mu_{\text{pcc}} = 0.18, \mu_{\text{hma}} = 0.35$
- Isotropic linear expansion
 - $\alpha_{\text{pcc}}, \alpha_{\text{hma}}$
- 20 node brick elements
(C3D20 quadratic element)



Foundation support

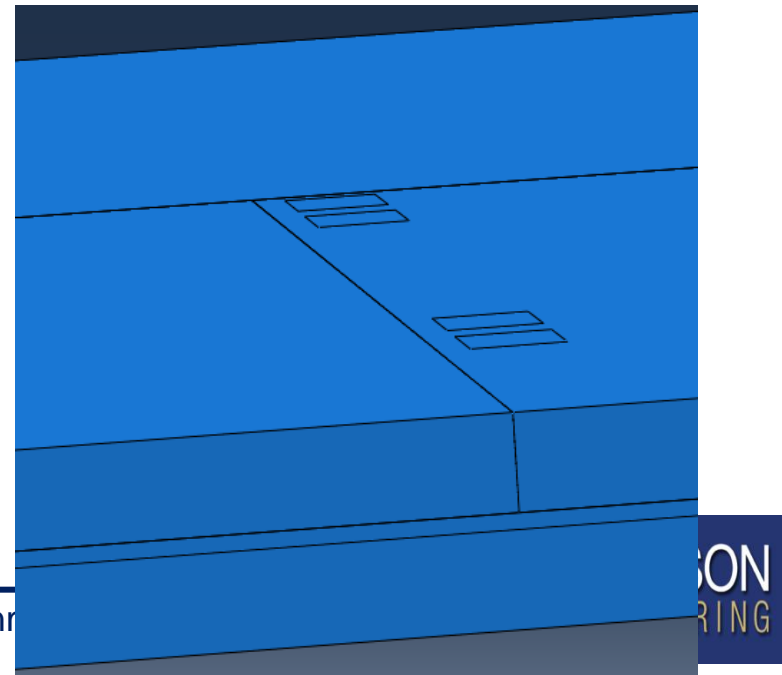
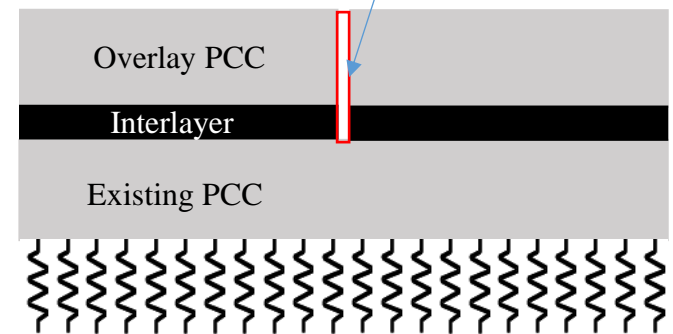
- Winkler foundation



Transverse joints

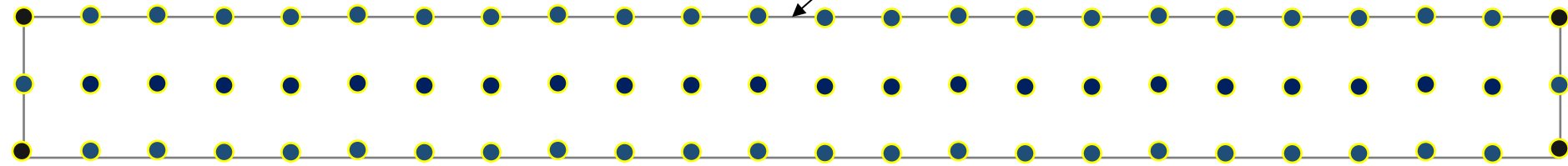
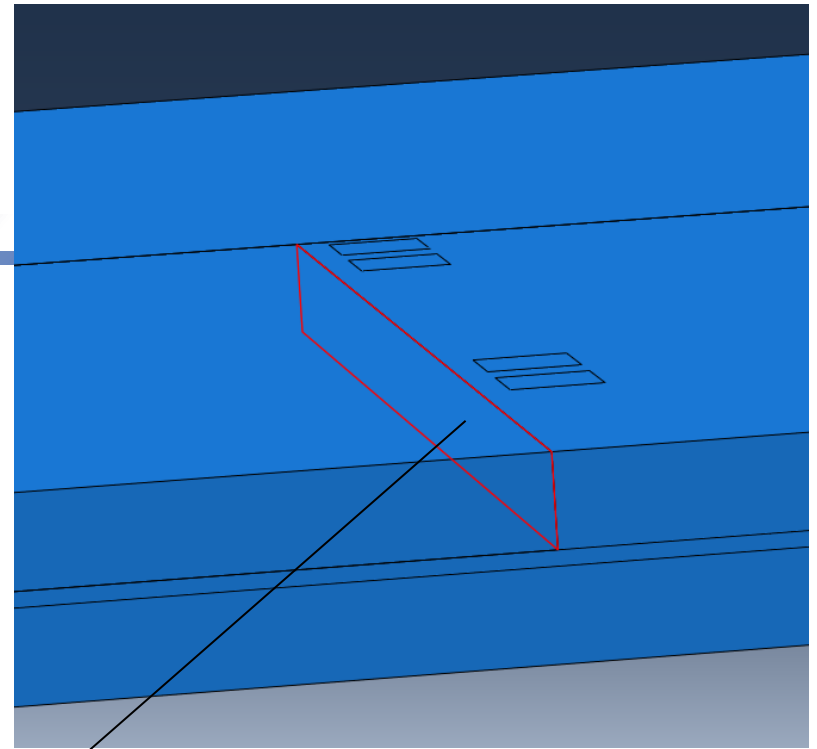
- Joint through overlay and asphalt
- Load transfer in PCC only
 - Shear springs at overlay nodes
 - No load transfer through interlayer
 - Only dof in vertical direction
 - Simulate aggregate interlock and doweled joints
- Hard contact between joint surfaces
 - simulate compression effects

Shear spring
for overlay
load transfer



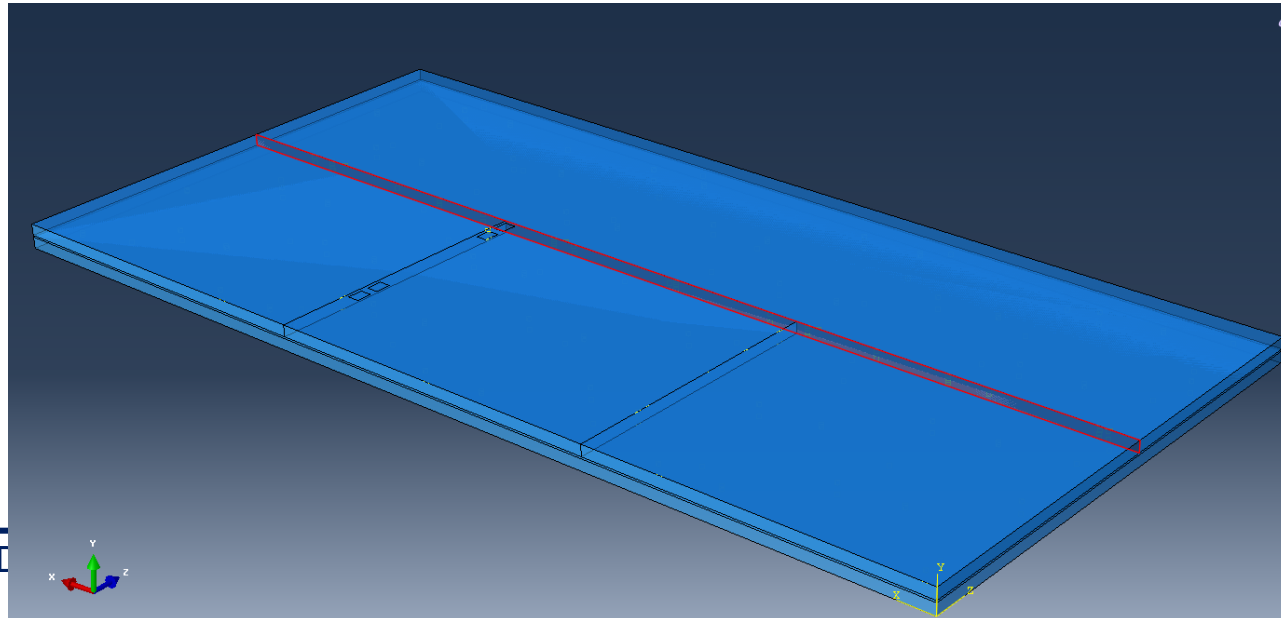
Transverse joints

- 3 spring stiffnesses
 - Corner nodes - K
 - Edge nodes - $2K$
 - Interior nodes - $4K$



Longitudinal joints

- Lane-shoulder joint
 - Asphalt shoulder – $LTE = 0\%$
 - Tied PCC shoulder – $LTE \sim 90\%$
- 6 ft x 6 ft longitudinal joint
 - Longitudinal $LTE =$ Transverse LTE (undoweled jt)
 - Longitudinal $LTE <$ Transverse LTE (doweled jt)

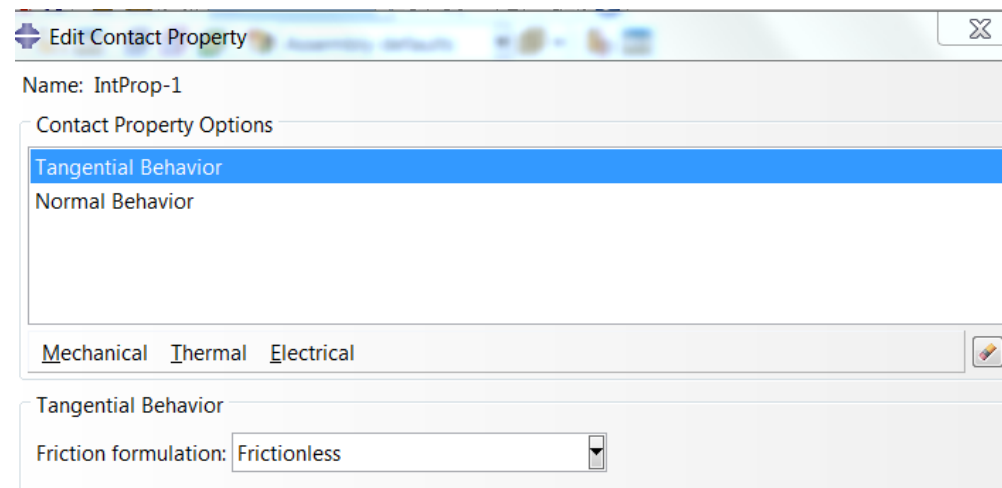
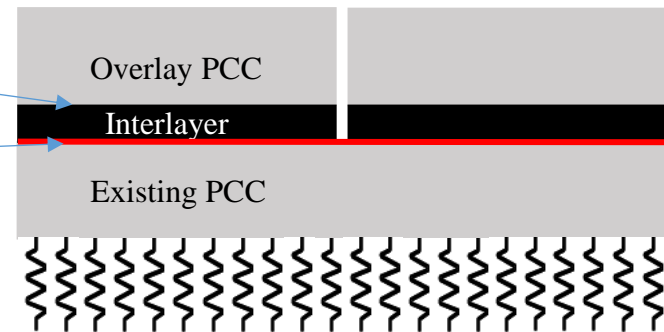


Interface bond: existing-interlayer

- Interaction between top of existing and interlayer:
- Treated as frictionless
- Full slip between two surfaces

Interface tied with
spring elements

Frictionless
interface

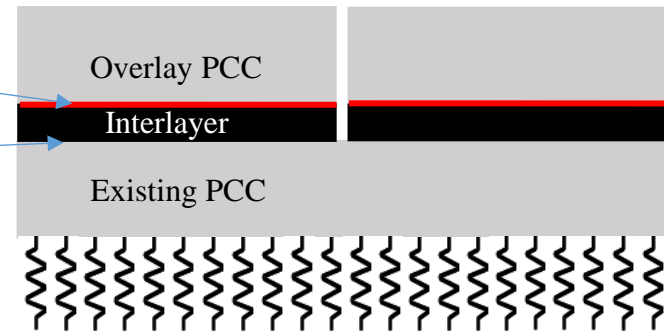


Interface bond: overlay-interlayer

- Interaction between overlay and interlayer:
 - Treated as fully bonded
 - Achieved with stiff springs connecting nodes of parts
 - Response can be modified to achieve debonding and to introduce gap between overlay and interlayer

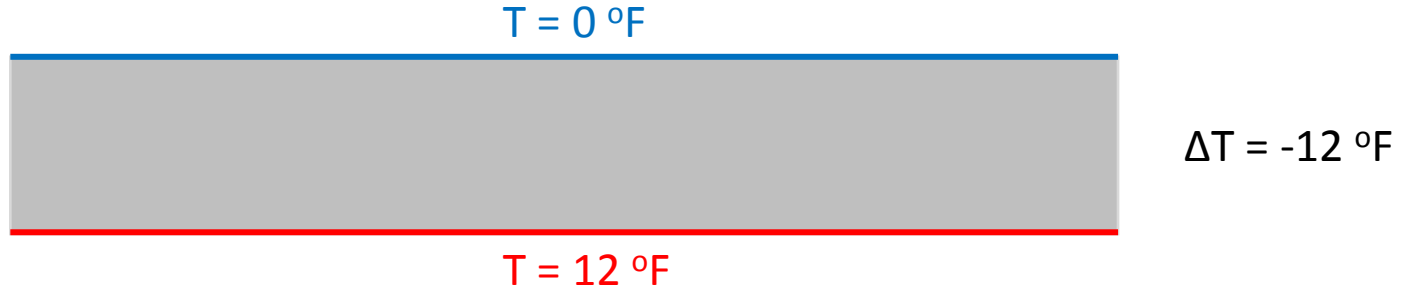
Interface tied
with spring
elements

Frictionless
interface



Thermal loads

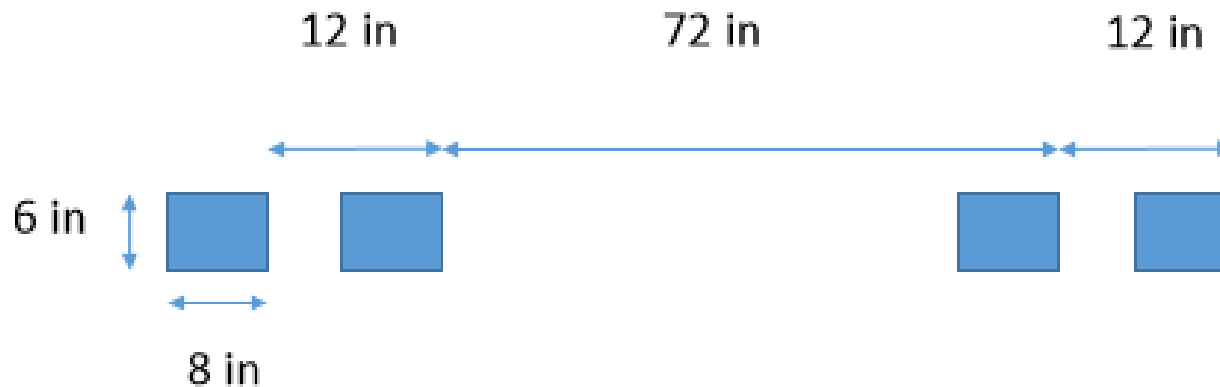
- Uniform distributed temperature loads
 - Predefined field on top/bottom PCC surfaces
 - 3 temperature differences considered
 - -12, 0, 24 °F



- Equivalent strain will be used to convert nonlinear temperature differences

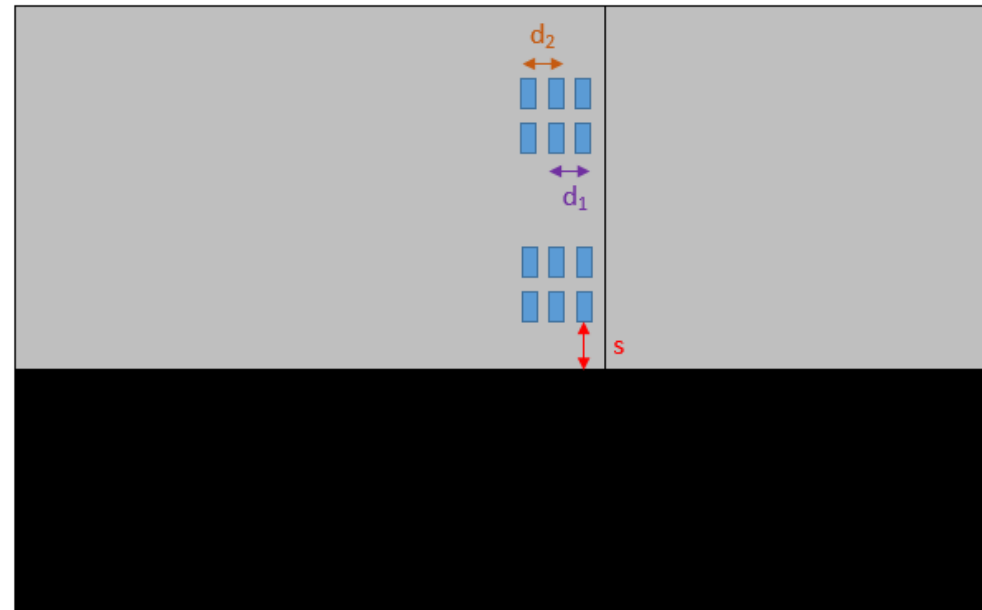
Axle configuration

- Tire footprint of 6 in x 8 in
- Single, tandem, and tridem
- Wheel wander



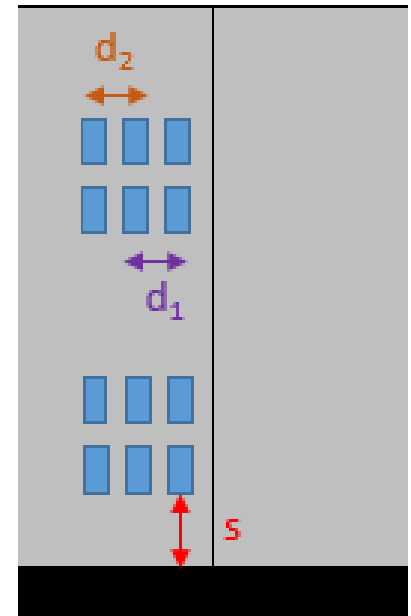
Axle type and wheel wander

- s = wheel wander
 - 0, 2, 6, 12, 36 in
- d_1 = tandem axle spacing
 - 0, 40 in
- d_2 = tridem axle spacing
 - 0, 40 in

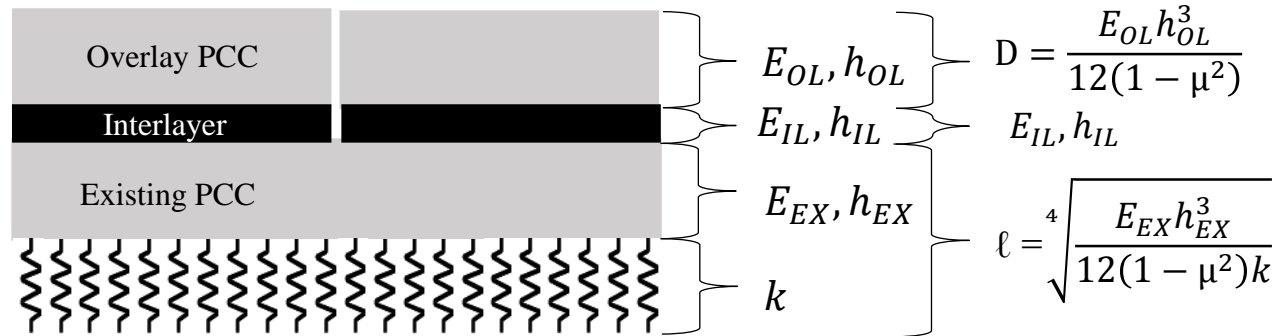


Load magnitude

- Single axles
 - 0 – 45,000 lbs (15,000 lb increment)
- Tandem axles
 - 0 – 90,000 lbs (30,000 lb increment)
- Tridem axles
 - 0 – 120,000 lbs (40,000 lb increment)
- Gravity load
 - Uniform pressure on surface
 - Equal to weight of structure



Reduction of parameters



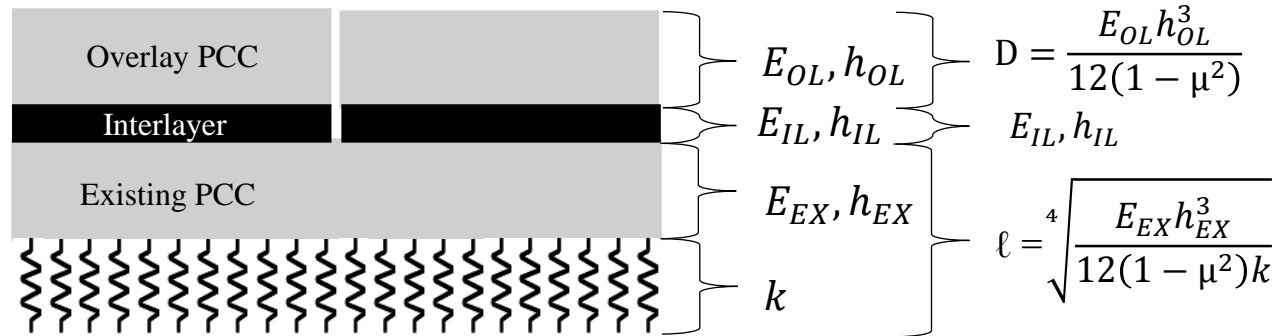
- Overlay represented w/ flexural stiffness, D

$$D = \frac{E_{OL} h_{OL}^3}{12(1 - \mu^2)}$$

- Existing PCC & foundation represented w/ radius of relative stiffness, ℓ

$$\ell = \sqrt[4]{\frac{E_{EX} h_{EX}^3}{12(1 - \mu^2)k}}$$

Reduction of parameters

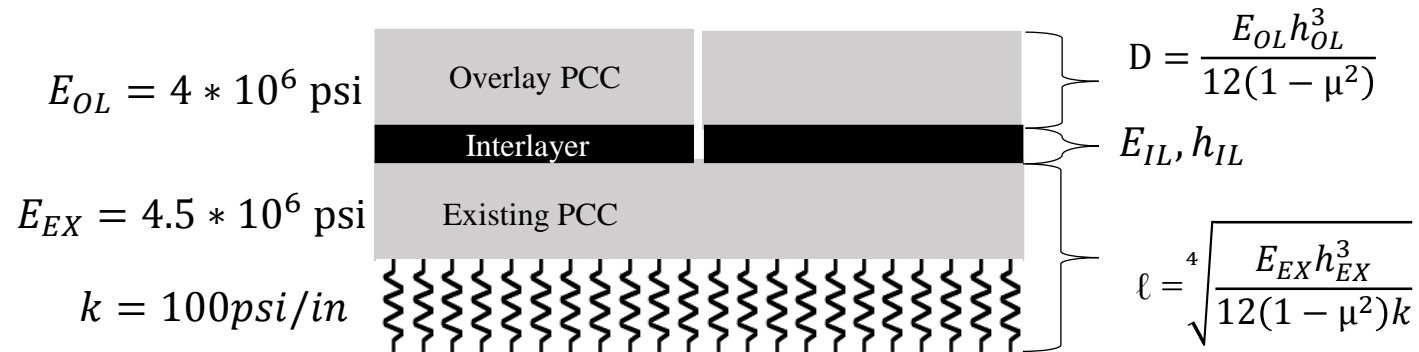


- PCC E and k-value kept constant
- Change h of PCC layers to change D_{OL} and ℓ

Range of parameters

Parameter	Range				
Existing slab and foundation, l (in)	20	35	50	65	80
PCC Poisson's ratio	0.18				
Overlay Flexural Stiffness, D (#-in)	2.00E+07	2.40E+08	4.60E+08	6.80E+07	9.00E+08
Overlay PCC jt spacing (ft)	6	10	15	20	
Overlay PCC CTE (in/in/°F)	3.80E-06	5.50E-06			
Overlay Temp Difference (°F)	-12	0	24		
Interlayer Thickness (in)	2				
Interlayer Stiffness (psi)	100000	400000	700000	1000000	
Interlayer Poisson's ratio	0.35				
Interlayer CTE (in/in/°F)	6E-06				
Lane shoulder LTE (%)	Tied PCC	Asphalt			
Wheel wander (in)	0	2	6	12	36
Single axle (lb)	0-45,000 (15 kip increment)			Fractional	
Tandem axle (lb)	0-90,000 (30 kip increment)			Factorial	
Tridem axle (lb)	0-120,000 (40 kip increment)				

Range of parameters



- D and ℓ ranges result in wide range of slab thicknesses considered:
 - $h_{OL} = 3.9 - 13.8 \text{ in}$
 - $h_{EX} = 3.5 - 22 \text{ in}$

Thank You

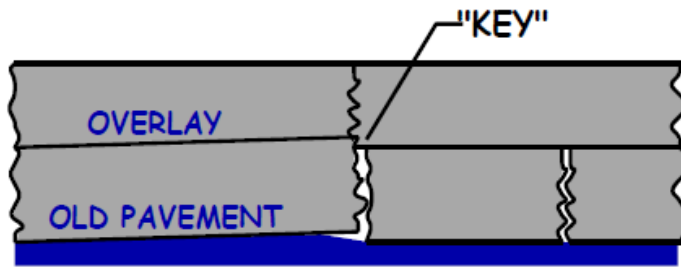


Any Questions?

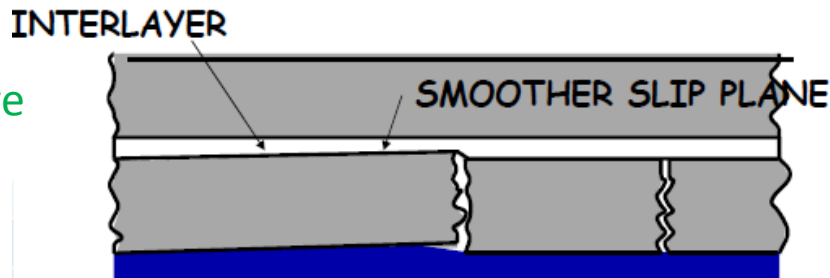
Keying in overlay

- How much faulting can be in existing pavement before nonwoven fabric no longer prevents keying?
 - Function of fabric thickness

Reflective
Cracking



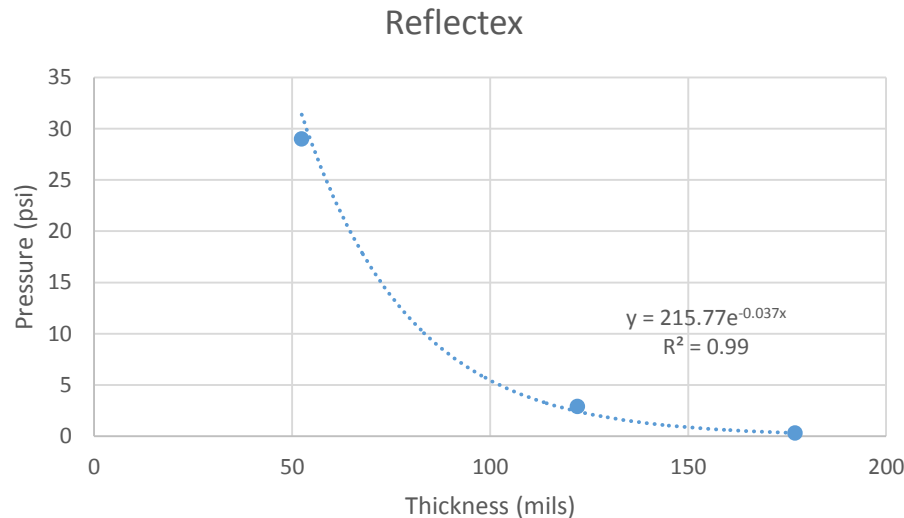
No reflective
Cracking



Photos courtesy of John Donahue of MoDOT

Reflectex fabric thickness

- ASTM D5199 – relate pressure to overlay thickness



Overlay Thickness (in)	Thickness (mils)
0.28	177
2.8	122
28	52.4

- Thickness due to self-weight of 6 in overlay ~ 96 mils