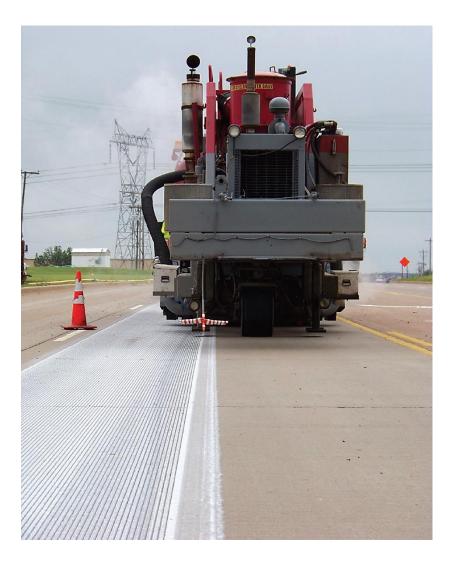


# **DEVELOPMENT OF SPS-2 PAVEMENT PRESERVATION EXPERIMENT**



## **FINAL REPORT** June 27, 2016

Washington State Department of Transportation 310 Maple Park Ave., SE Olympia, Washington 98504

# **Development of SPS-2 Pavement Preservation Experiment Final Phase I Report**

Pooled fund TPF -5(291)

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Final Phase I Report Pooled Fund Study TPF-5(291) Development of an SPS-2 Pavement Preservation Experiment

## **Development of an SPS-2 Pavement Preservation Experiment**

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#### **EXECUTIVE SUMMARY**

The Long Term Pavement Performance (LTPP) SPS-2 experiment - strategic study of structural factors for rigid pavements - is the most comprehensive on-going concrete pavement research effort undertaken since the AASHO Road Test. Spanning fourteen states, the study begin in 1992 and eleven of the original fourteen sections remain in service with current years of service ranging between 14 and 22 years as of 2015. Given this unparalleled resource of well documented and monitored aged concrete pavements, the sections currently in study provide the ideal opportunity to develop a second experiment to compare the effectiveness of concrete pavement preservation strategies to extend pavement service life. Recognizing the opportunity, this pooled fund study (TPF-5(291)) was initiated to develop and implement a continuation experiment focused on pavement preservation. As a precursor to the full experiment, the evaluation and assessment of the existing SPS-2 sections with current data limitations and availability must be analyzed and discussed in order to proceed with the development of a robust experimental plan.

This report outlines the current availability of LTPP data, including sections remaining in study, and of those, which have received maintenance or rehabilitation treatments that may limit the options of inclusion in further study. Pavement preservation techniques are evaluated and discussed, including limitations that should be considered due to site specific factors or test section history. The available test sections were considered with the respective potential limitations based on the investigated pavement preservation methods. Multiple options of pairing the test sections for evaluating a control and a testing section are presented and several potential experiments are identified to investigate the performance of spall repairs, joint sealing, diamond grinding, crack sealing, and the combination maintenance of joint sealing and diamond grinding.

Additionally, an experiment to investigate the effectiveness of dowel bar retrofit is presented that utilizes the original supplemental sections, some of which were originally undoweled. This would allow for comparing the variation in load transfer efficiency and difference in faulting between a previously undoweled section receiving a dowel bar retrofit and sections with dowels that will receive diamond grinding for maintenance.

The proposed experiment could be expanded to include using the wealth of existing pavement performance data from the SPS-2 experiment to utilize the predicted performance curves produced from AASHTOWARE PavementME as the "control" sections. If successful, this would eliminate paired sections and allow for doubling of the experimental sections. Additional work will be necessary to establish the validity of this approach before revising the experimental plan. This report for Phase I summarizes the work completed thus far evaluating and recommending possible experiments based on existing data and the pooled fund panel will be evaluating and guiding the future direction of the project.

#### BACKGROUND

The Long Term Pavement Performance (LTPP) SPS-2 experiment, strategic study of structural factors for rigid pavements, is the most comprehensive on-going concrete pavement research effort undertaken since the AASHO Road Test. Spanning fourteen states, the study begin in 1992 and eleven of the original fourteen sections remain in service with life spans ranging between 14 and 22 years as of 2015. Given this unparalleled resource of well documented and monitored aged concrete pavements, the sections currently in study provide the ideal opportunity to develop a second experiment to compare the effectiveness of concrete pavement preservation strategies to extend pavement service life. Due to the age of the sections, the timeframe of establishing this continuation experiment is critical before the sections in study will require any additional maintenance to maintain adequate serviceability. To begin addressing this project, the pooled fund study (TPF-5(291)) was initiated to develop and implement this continuation experiment. As a precursor to the full experimentation, the evaluation and assessment of the existing SPS-2 sections with current data limitations and availability must be analyzed and discussed in order to proceed with the development of a robust experimental plan.

This work was completely funded by the Transportation Pooled Fund study program, initiated by the FWHA. Washington served as the lead state for this project and supporting states included in the pooled fund included Arizona, California, Colorado, Georgia, Kansas, and North Carolina. The first project initiation meeting occurred on January 6, 2014, and the first panel meeting was held on March 11, 2016.

#### INTRODUCTION

The SPS-2 experiments were designed to investigate the effect of several key factors on the performance of doweled jointed plain concrete pavement. These factors were selected based on input provided from participating state and provincial highway agencies and used to develop the core experiment, which will be discussed in more detail below. The core experiment construction requirements were held consistent across all participating states. States were then given the option to include supplemental test sections of interest to the State Highway Agency (SHA). The original intent was to develop a robust secondary experiment with the supplemental test sections. However, states indicated a preference toward designing individualized, state-specific experiments. The supplemental sections were not held consistent across the participating SHAs but contained some similar factors across different states, including testing dowel bar effectiveness, joint spacing and skew. The organization and implementation of the supplemental test sections will also be discussed in more detail in this section.

#### **Core Experiment**

As previously discussed, the SPS-2 core experiment was comprised of both site specific and structural factors that were based on the interest and input of participating agencies. The factors considered are summarized in Table 1 below. Factors were divided between site related factors (environmental) and structural factors. There were three site-specific factors that were originally considered experimentally significant: traffic, climate, and subgrade. However, traffic was not included in the experimental design matrix and rather, a minimum level of traffic was required for sites to be considered eligible for participation. This lead to varying traffic levels across test sites that will be discussed later in the report. The other site related factors included climate, which was divided into four levels based on climatic zones: wet-freeze, wet-no freeze, dry-freeze, and dry-no freeze, and two subgrade factor levels of either fine or coarse subgrade. There were five structural factors: base type, drainage type, concrete thickness, concrete flexural strength, and lane width. The levels considered in the experiment are also presented in Table 1 below. It should be noted that some states were unable to achieve the exact requirements and some variation does exist between some required factors in the test sections.

Type of Experimental factor	Variables affected	Experimental factor	Number of levels	Levels
		Traffic	0	N/A
				Wet-Freeze
	Site-specific	Climate	4	Wet-No Freeze
Environmental		Climate	4	Dry-Freeze
				Dry-No Freeze
		Subarado	2	Fine
		Subgrade	Z	Coarse
				Dense-graded untreated unbound aggregate (DGAB)
	Base/Subbase	Base type	3	Lean concrete (LCB)
				Open graded permeable asphalt stabilized base (PATB)
		Drainage type	2	Open graded permeable asphalt drainage layer (PATB)
Structural				No drainage layer
		PCC thickness	2	8 inch
		PCC INICKNESS	2	11 inch
	Pavement	PCC flexural	2	550 psi
	surface	strength	Z	900 psi
		Lane width	2	12 ft
			۷.	14 ft

Table 1. Experimental factors considered in the original SPS-2 experiment

These factors were then compiled into a factorial experiment which, between the eight environmentally related factors and 24 pavement related structural factors, resulted in 192 factor level combinations that would require 24 test sections constructed at each site for a full factorial experiment. Due to the financial and energy intensity of this endeavor for participating agencies, the resulting constructed experiment was a half-factorial experiment that coupled the full factorial design based on climatic sub-zone. This resulted in only 12 test sections to be constructed at each site. The initial ideal experimental design table is shown in Table 2 below based on the finalized half-factorial experiment. Then the actual construction matrix based on available agencies is given in Table 3 to reflect actual SPS-2 construction.

	Paver	nent Stru	ucture							C	Climate	zones	, subgi	rade sit	te						
			PCC					W	/et							D	ry				
Drainage	Base		PCC	Lane		Fre	eze			No Fi	reeze		Freeze No Freeze								
Drainage	Туре	Thick,	Strength,	Strength,	Width	Fi	ne	Co	arse	Fi	ne	Coa	arse	Fi	ne	Coa	arse	Fi	ine	Соа	arse
		in	psi		J	К	L	М	Ν	0	Р	Q	R	S	Т	U	V	W	Х	Y	
		550	12	J1		L1		N1		P1		R1		T1		V1		X1			
		8	14		K13		M13		013		Q13		S13		U13		W13		Y13		
		0	900	12		K14		M14		014		Q14		S14		U14		W14		Y14	
No	No DGAB		500	14	J2		L2		N2		P2		R2		T2		V2		X2		
110	DOND	550	12		K15		M15		015		Q15		S15		U15		W15		Y15		
			14	J3		L3		N3		P3		R3		Т3		V3		X3			
			900	12	J4		L4		N4		P4		R4		T4		V4		X4		
				14		K16		M16		016		Q16		S16		U16		W16		Y16	
		550	12	J5		L5		N5		P5		R5		T5		V5		X5			
		8		14		K17		M17		017		Q17		S17		U17		W17		Y17	
	No LCB –	_	900	12		K18		M18		018		Q18		S18		U18		W18		Y18	
No				14	J6		L6		N6		P6		R6		T6		V6		X6		
			550	12		K19		M19		019		Q19		S19		U19		W19		Y19	
		11		14	J7		L7		N7		P7		R7		T7		V7		X7		
			900	12	J8	1/20	L8	1420	N8	020	P8	0.20	R8	620	Т8	1120	V8	14/20	X8	2/20	
				14	10	K20	10	M20	NO	020	P9	Q20	50	S20	Т9	U20	10	W20	VO	Y20	
			550	12	19	1/21	L9	N421	N9	021	P9	021	R9	C24	19	1124	V9	14/21	X9	Y21	
		8		14 12		K21 K22		M21		021 022		Q21 Q22		S21 S22		U21 U22		W21 W22		Y21 Y22	
	PATB/	900	12	J10	KZZ	L10	M22	N10	022	P10	QZZ	R10	522	T10	022	V10	VVZZ	X10	YZZ		
Yes	DGAB		14	110	K23	LIU	M23	NIU	023	P10	Q23	RIU	S23	110	U23	V10	W23	X10	Y23		
	DGAB		550	12	J11	N23	L11	11/23	N11	023	P11	Q23	R11	323	T11	023	V11	vvz3	X11	123	
		11		14	J11 J12		L11 L12		N11 N12		P11 P12		R11 R12		T11		V11 V12		X11 X12		
			900	12	JTZ	K24	LIZ	M24	INTZ	024	P12	Q24	K12	S24	112	U24	VIZ	W24	X12	Y24	
				14		KZ4		10124		024		QZ4		524		024		VV 24		124	

# Table 2. Experimental matrix for half-factorial experiment design for SPS-2 experiment.

PCC Thickness, in 8	Base Type Thickness, in	Flexural strength, 14-d (psi) 550	Lane width, ft 12	ОН	Fine	Free		Wet		N						Dry							
Thickness, in 8	Base Type Thickness, in	strength, 14-d (psi)	width, ft 12	ОН		Free				NI - 1									Dry				
in 8	in in	14-d (psi)	ft 12	ОН				Freeze				No-Freeze			Freeze			No-Freeze					
8			12		1.0			Coarse		Fine	Coarse	Fine			Coarse	Fine	Coa	irse					
	8	550			IA	MI	DE	AR	WI	NC		KS	WA	ND	CO	NV		CA	AZ				
	8			1			1			1		1	1			1		1					
			14		13	13		13	13					13	13				13				
		900	12		14	14		14	14					14	14				14				
	DGAB		14	2			2			2		2	2			2		2					
		550	12		15	15		15	15					15	15				15				
11	11	11 900	14	3			3			3		3	3			3		3					
			12	4			4			4		4	4			4		4					
			14	_	16	16		16	16				_	16	16			_	16				
		550		5	47	47	5	47	47	5		5	5	47	47	5		5	47				
8	8	900															-		17				
				6	18	18	<u> </u>	18	18	6		6	6	18	18	6			18				
	LCB			6	10	10	6	10	10	6		6	6	10	10	6		6	19				
		550		-	19	19	7	19	19	7		7	7	19	19			-	19				
11	11									-		-	-				-						
		900		8	20	20	8	20	20	8		8	8	20	20	8		8	20				
				0	20	20	0	20	20	0		0	0	20	20	0			20				
		550		9	21	21	9	21	21	9		9	9	21	21	9		9	21				
8	8																		21				
-B 900			10	22	22	10	22	22	10		10	10	22	22	10		10	22					
	РАТВ		10	22	22	10	22	22	10		10	10	22	22	10		10	23					
		550		11	25	25	11	25	25	11		11	11	25	25	11		11	25				
11	11																		┝──┤				
		900			l .					14		12	14	24	24				24				
		8	8         900           11         550           900         550           8         900           11         550           550         550           8         900           11         550	$\begin{array}{c c} & & & 12 \\ & & & 14 \\ \hline 8 & & & \\ \hline 900 & & & 12 \\ \hline 900 & & & 14 \\ \hline 11 & & & \\ \hline 550 & & & 12 \\ \hline 14 & & \\ \hline 900 & & & & 12 \\ \hline 8 & & & & \\ \hline 11 & & & & \\ \hline 11 & & & & \\ \hline 900 & & & & & \\ \hline 12 & & & & \\ \hline 11 & & & & \\ \hline 900 & & & & \\ \hline 12 & & & \\ \hline 11 & & & \\ \hline 900 & & & & \\ \hline 12 & & & \\ \hline 12 & & & \\ \hline 12 & & & \\ \hline 11 & & & \\ \hline 12 & & & \\ \hline 12 & & & \\ \hline 11 & & & \\ \hline 12 & & & \\ 12 & & & \\ \hline 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & & \\ 12 & & \\ 12 & & & \\ 12 & & \\ 12$	$\begin{array}{c cccc} & 12 & 5 \\ & 14 & \\ \hline & & \\ 900 & 12 & \\ \hline & & \\ 900 & 14 & 6 \\ \hline & & \\ 11 & 550 & 12 & \\ \hline & & \\ 11 & & \\ 900 & 12 & 8 \\ \hline & & \\ 900 & 14 & \\ \hline & & \\ 11 & & \\ 900 & 12 & \\ \hline & & \\ 11 & & \\ 900 & 12 & 12 \\ \hline & & \\ 11 & & \\ 900 & 12 & 12 \\ \hline \end{array}$	$\begin{array}{c ccccc} & 12 & 5 & \\ \hline & & 14 & & 17 \\ \hline & & & & 12 & & 18 \\ \hline & & & & & & & \\ \hline & & & & & & \\ \hline & & & &$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$								

 Table 3. As-Constructed experimental matrix for half-factorial experiment design for SPS-2 experiment.

It can be seen from Table 3 that the experiment as constructed resulted in the matrix being unable to be exactly filled due to agency availability and participation. For example, there were no experiments with fine subgrade in a no-freeze dry climate and no experiments with a coarse subgrade in a no-freeze wet climate. Only a single, non-coupled experiment exists for fine soil in a no-freeze wet climate and likewise only one single, non-coupled experiment was constructed for coarse soil in a dry freeze climate. Also again due to agency participation, some cells from the matrix can be seen as replicates.

#### Supplemental Experiments

Originally, the supplemental sections were intended to formally create several robust, secondary experiments regarding several additional factors of interest as determined by state agencies including the use of dowel bars, joint spacing, and joint skew. However, during development, it was determined that states preferred more agency-specific supplemental experiment options. This resulted in some variation of experiments by state, including some states that did not have any supplemental sections or constructed only a single supplemental section, which was often constructed with the standard concrete paving mix and structure for the agency.

#### Arizona

The supplemental sections in Arizona were divided into three smaller experiments that were designed to investigate random skew joints, slab thickness, and project variability, respectively. To investigate random, skew joints, the concrete mix design was held consistent across this experiment, and the lower strength mix (550 psi flexural strength) used in the core experiments was also used in this supplemental experiment. The joints for all slabs were skewed by two feet across the width with the pattern 13, 15, 17, 15 ft and all slabs were undoweled. This experiment also utilized a half-factorial design and investigated three factors: slab width, base type, and slab thickness. The base types considered included dense graded aggregate base (DGAB) and permeable bituminous treated base (PBTB).

The second state experiment varied the concrete slab thickness and was constructed over a Bituminous Treated Base (BTB) mix and all sections were doweled with 15 ft joint spacing. Again, the lower strength mix used in the core experiments (550 psi) was used consistently across these supplemental sections.

Finally, the third smaller experiment investigated site-based variability and the two identical AC test sections were constructed on either end of the site. This allowed for comparison across the site to ensure consistency across all of the sites. Site-specific information is given in Table 4 below.

18

SHRP ID	Sub- experiment	Lane width, ft	Base type	PCC thickness, in	Dowels			
040262		14	DGAB	8	No			
040263	1	14	PBTB	8	No			
040264		12	PBTB	11	No			
040265		12	DGAB	11	No			
040266		14	BTB	12.5	Yes			
040267	2	14	BTB	11	Yes			
040268		14	BTB	8	Yes			
040260	3	(intentionally identical and on either end of the project;						
040261	3	asphalt surface)						

Table 4. Supplemental sections constructed for the Arizona SPS-2 experiment.

## Delaware

Delaware constructed two supplemental test sections to investigate the effect of different dowel bars on concrete pavement performance. The two sections, whose properties are given in Table 5 below, are identical except for the type of dowel used.

SHRP ID	Lane width, ft	Compressive strength, psi	DGAB thickness, in	PCC thickness, in	Dowel type
100259	12	3000	8	10	steel
100260	12	3000	8	10	plastic

Table 5. Supplemental sections constructed for the Delaware SPS-2 experiment.

## North Dakota

North Dakota constructed six supplemental sections for the SPS-2 experiment. This included one control section, constructed to the standard methods of practice for NDOT (380259) and five additional experimental sections. These sections investigated several factors: the inclusion of dowels (only 380260 was doweled), the use of skewed joints at varying lengths (380261, 380262, and 380263 are all spaced with variable joint spacing alternating on the same pattern of 12, 15, 13, and 14 ft) and finally, base type was also varied across sections. The details of all supplemental sections constructed in North Dakota are given in Table 6 below.

SHRP ID	PCC Pavement Thickness (in)	Joint orientation	Strength	PCC Pavement width, ft	Base Type	Dowels	Joint spacing
380260	11	Skewed	***	38	DGAB	Yes	15 ft
380261	11	Skewed	550	24	DGAB	No	Variable**
380262	11	Skewed	550	28	LCB	No	Variable**
380263	11	Skewed	550	24	PASB	No	Variable**
380264	11	Skewed	***	38	PASB	No	15 ft
380259*	10	Skewed	***	24	8" Salve	Yes	15 ft

Table 6. Supplemental sections constructed for the North Dakota SPS-2 experiment.

\* indicates state control section which used the standard state mix design

\*\* "variable" indicates joint spacing varying from 12, 15, 13, and 14 ft

\*\*\* considered Class AE concrete as per NDDOT specifications

## Ohio

The Ohio supplemental sections were constructed to test several factors including base type and thickness, AB thickness, and the mix design as shown in Table 7 below. Two different base types were compared: permeable asphalt treated base (PATB) and cement treated free draining base (CTFDB). Sections 390259, 390263, and 390264 were not constructed on an unbound aggregate base, but constructed on 6 inches of asphalt base, while sections 390260 and 390265 were both constructed on PATB and sections 390261 and 390262 were constructed on CTFDB. The PCC thickness was held constant across all supplemental sections. The supplemental sections were alternated between low and high strength mix designs. Details of the construction of the Ohio supplemental sections are given in Table 7 below while details of the two mix designs used are given in Table 8.

Table 7. Supplemental sections constructed for the Ohio SPS-2 experiment.

SHRP ID	PCC thickness	AB, in	Base type	Base thickness, in	Mix design
390259	11	6		0	А
390260	11	4	PATB	4	В
390261	11	4	CTFDB	4	А
390262	11	4	CTFDB	4	В
390263	11	6		0	А
390264	11	6		0	В
390265	11	4	РАТВ	4	А

	Mix des	sign, lbs/CY
	Mix A	Mix B
Coarse Aggregate	1680	1850
Fine Aggregate	1260	950
Cement	510	750
Water	240	270
Fly Ash	90	113
Air entrainer	7.2-9.6	8-12.7 oz
	OZ	8-12.7 02
Water reducer	18 oz	26.3-36.8
water reducer	10.02	OZ

Table 8. Mix design detail for supplemental sections constructed for the Ohio SPS-2 experiment.

#### Wisconsin

Wisconsin constructed eight supplemental sections, including two identical control sections of 550259 and 550260. Several factors were varied across the other test sections including lane width, subbase thickness, rock base thickness, embankment fill thickness, and PCC strength. The details of the differentiation between all test sections is given in Table 9 below.

Table 9. Supplemental sections constructed for the Wisconsin SPS-2 experiment.
--------------------------------------------------------------------------------

SHRP ID	Lane width, ft	Subbase thick., in	Rock base thick., in	Embankment fill thick., in	DGAB thick., in	CSOGB thick., in	PCC thick., in	PCC strength, psi
550259*	14			24	6		11	550
550260*	14			24	6		11	550
550261	12			24	4	4	8	550
550262	12	10	3		6		8	900
550263	14			24	6		10	550
550264	14			24	6		11	550
550265	14	10			6	4	11	550
550266	14			24	6		11	

(\*) indicates control section

## States with control sections only

Seven additional states constructed only a "control" section in addition to the core experiment, rather than a series of supplemental sections. These control sections were constructed in accordance

with each state's specific pavement construction requirements. Certain aspects of the construction varied across the control sections and pertinent construction and material details as given by each agency are compiled in Table 10 below.

		Lane	PCC		Base	PCC	
SHRP	State	width,	thickness,	Base type	thickness,	Strength,	Notes
		ft	in		in	psi	
080259	CO	12	11	None	0	650	
190259	IA	14	11	Granular base	6		
200259	KS	12	12	Stabilized base/modified fly ash	6	600	
260259	MI	12	11	Granular base, treated base	4, 4	550	Has tied concrete shoulders and neoprene transverse joints and hot poured rubberized sealant longitudinal joints
32059	NV		11	Treated base	1.5		
370259	NC	12	10	Permeable Asphalt Treated Base	5		
530259	WA	14	10	ATB/crushed surfacing base course (CSBC)	3, ATB 2, CSBC	650	

Table 10. Control sections constructed by state.

## **CURRENT DATA AVAILABILITY**

The availability of specific test data was evaluated as part of Tasks 1 and 2, in the original project description. The existing data was evaluated to discern which sites remained in service and have been compiled and presented in Table 11. The shaded cells indicate sites that have been removed from study. No sites from the SPS-2 experiment were reassigned to a different experiment; therefore, sites removed from study indicate that data is no longer being actively collected. Table 11 presents the updated original experiment matrix indicating which sites remain in study of the core experimental sections.

		avement Struc	cture			Climatic Conditions and Subgrade							
								V	/et				
Drainago	Base	PCC	Flexural	Lane				No Freeze					
Drainage	Туре	Thickness, in	strength, 14-d (psi)	width		Fine			Coarse		Fine	Coarse	
					ОН	IA	MI	DE	AR	WI	NC		
			550	12	390201			100201			370201		
		8	550	14		190213	260213		050213	550213			
		ŏ	900	12		190214	260214		050214	550214			
No	DGAB		900	14	390202			100202			370202		
NO	DGAD		550	12		190215	260215		050215	550215			
		11		14	390203			100203			370203		
			000	12	390204			100204			370204		
			900	14		190216	260216		050216	550216			
		8		550	12	390205			100205			370205	
			550	14		190217	260217		050217	550217			
			900	12		190218	260218		050218	550218			
Nie				14	390206			100206			370206		
No	LCB	11	550	12		190219	260219		050219	550219			
			550	14	390207			100207			370207		
			000	12	390208			100208			370208		
			900	14		190220	260220		050220	550220			
			550	12	390209			100209			370209		
		0	550	14		190221	260221		050221	550221			
		8	000	12		190222	260222		050222	550222			
N.s.s	DATD		900	14	390210			100210			370210		
Yes	ΡΑΤΒ		550	12		190223	260223		050223	550223			
			550	14	390211			100211			370211		
		11	0.00	12	390212			100212			370212		
			900	14		190224	260224		050224	550224			

Table 11. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections.

		vement Struc				•		Conditions							
		500	Flexural					Dry							
Drainage	Base	PCC Thickness,	strength,	Lane		Freeze					No Freeze				
Dialilage	Туре	in	14-d	width		Fii	ne		Coarse	Fine	Coa	arse			
			(psi)		KS	WA	ND	CO	NV		CA	AZ			
				550	12	200201	530201			320201		060201			
		8	550	14			380213	080213				040213			
		0	900	12			380214	080214				040214			
No	DGAB		900	14	200202	530202			320202		060202				
No	DGAD	11	550	12			380215	080215				040215			
			550	14	200203	530203			320203		060203				
			000	12	200204	530204			320204		060204				
			900	14			380216	080216				040216			
			550	12	200205	530205			320205		060205				
		8	550	14			380217	080217				040217			
		0	900	12			380218	080218				040218			
Ne				14	200206	530206			320206		060206				
No	LCB		550	12			380219	080219				040219			
		11	550	14	200207	530207			320207		060207				
		11	900	12	200208	530208			320208		060208				
						900	14			380220	080220				040220
			550	12	200209	530209			320209		060209				
		0	550	14			380221	080221				040221			
		8	000	12			380222	080222*				040222			
			900	14	200210	530210			320210		060210				
Yes	ΡΑΤΒ		550	12			380223	080223				040223			
			550	14	200211	530211			320211		060211				
		11	0.5.5	12	200212	530212			320212		060212				
			900	14			380224	080224				040224			

Table 11. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections (continued)

It can be seen from Table 11 that of the original fourteen states participating in the SPS-2 study, each with twelve test sections, eight remain completely intact (Arizona, California, Delaware, Iowa, Kansas, North Dakota, Washington, and Wisconsin) with all twelve original sections still in study. Two of the original fourteen sites (Arkansas and Colorado) have only one out of the original twelve test sections removed from study resulting in eleven monitored test sections. Two of the original fourteen sites (North Carolina and Ohio) have half of the original test sections removed from study resulting in six test sections remaining in study in each state. Finally, all test sections from two of the original fourteen sites (Michigan and Nevada) have been removed from study.

The supplemental sections were also evaluated for current status. Table 12 below is a listing of the state and SHRP ID only with shaded cells indicating sites that are no longer in study. Please reference the previous section's discussion for the specific details regarding experiment design considerations for the supplemental sections.

Table 12. Summary of current status of SPS-2 experiment supplemental sections.
--------------------------------------------------------------------------------

	•
State	SHRP ID
State	040260
	040261
	040262
	040263
Arizona	040264
	040265
	040266
	040267
	040268
Colorado	080259
Deleurere	100259
Delaware	100260
lowa	190259
Kansas	200259
Michigan	260259
Nevada	320259
North Carolina	370259
	380259
	380260
Nouth Dolusto	380261
North Dakota	380262
	380263
	380264
	390259
	390260
	390261
Ohio	390262
	390263
	390264
	390265
Washington	530259
~	550259
	550260
	550261
	550262
Wisconsin	550263
	550264
	550265
	550266

It can be seen that most of the state supplemental sections remain in study. Only four of the forty total constructed supplemental sections have been removed from study since construction. This

provides ample possible testing sections; however, the widely varying design and construction parameters as discussed in the previous section makes the inclusion of these sections in the proposed study challenging with regards to providing appropriate means of comparison.

#### **PREVENTIVE MAINTENANCE METHODS**

Methods of pavement preservation are of utmost importance to highway agencies in order to extend the effective life of constructed pavement for economic, sustainability, and logistical reasons. Previously, the LTPP SPS-4 experiment, Preventive Maintenance Effectiveness of Rigid Pavements, was designed to address this concern, albeit more than 20 years ago. Preventive maintenance strategies, technologies, and materials have advanced greatly since then indicating a need for a more robust and updated research approach. Increasingly, the implementation of pavement preservation and maintenance has shifted from addressing existing distresses to a proactive, preventive approach. The terms preservation, rehabilitation, and preventative maintenance are best illustrated visually in Figure 1 below, reproduced from Smith et al (2014).

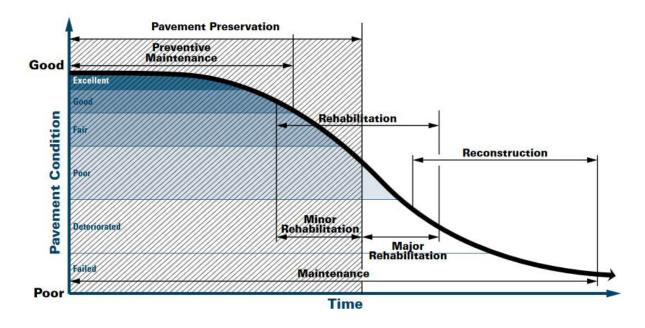


Figure 1. Visual representation of pavement preservation, maintenance, and reconstruction. Reproduced from Figure 2.1 of Smith et al (2014).

By compiling agency input as well as considering the experimental feasibility of some techniques, the following pavement preservation techniques for rigid pavements were selected to be included in the study.

- Partial depth patching (including spall repair)
- Dowel bar retrofit
- Joint sealing
- Diamond grinding

Partial depth repairs are a pavement preservation technique that involve removing and replacing areas of the pavement between ½ and 1/3 the full depth of the pavement slab. This can be an extremely effective method of repair for myriad distresses and are roughly divided into three categories based on the targeted areas of distress: joint repairs, non-joint repairs, and bottom half repairs. Joint repairs include removing the top portion of the joint and are ideally used for spalled joints or otherwise damaged joints. Repairs that occur away from the joint can also be due to spalling or cracking. Finally, bottom-half repairs are most commonly used for corner breaks or other corner-concentrated deterioration. For any of these repairs, the type of repair material used is extremely important to the effectiveness of this repair type and therefore a high quality concrete is often used (Smith et al 2014).

Dowel bar retrofits involve inserting dowel bars into existing joints to increase the load transfer efficiency across slabs. Since all sections in the core experiment contain dowel bars, this would only be effective to compare performance between undoweled supplemental sections and doweled sections. While aggregate interlock can provide some level of load transfer across joints, as joints age and widen, the effectiveness of aggregate interlock decreases which can merit the use of a dowel bar retrofit. Ultimately, improving the load transfer efficiency across joints can reduce the occurrence of pumping, faulting, and corner breaks.

Joint sealing is arguably the most truly preventive of all the maintenance options and includes removing the existing joint seal and replacing it. This prevents distresses related to the infiltration of incompressible materials, which can lead to joint or crack deterioration. The sealing does require maintenance, however, as the joint or crack can widen with age, which can stretch the joint filling material more than the crack width.

Diamond grinding is a technique utilizing diamond saw blades to remove a predetermined amount of surface to improve surface texture and smoothness. Grinding is used to reduce faulting, rutting, built in slab curl or warp, and to improve texture on polished aggregate surfaces.

Each of these potential repairs can treat one or several distresses as shown in Table 13 below, reproduced from Hall et al (2001) which details the types of distresses that can be addressed from each of these rehabilitation and maintenance treatments.

			Concre	te pavem	ent rehabili	tation techr	niques		
Concrete pavement distresses	Full depth repair/slab replacement	Partial depth repair	Slab jacking	Dowel bar retrofit	Joint resealing	Diamond grinding	Grooving	Pressure relief joints	Crack sealing
Corner break	×								
Linear cracking	×								×
Punchout	×								
Durability cracking	×								
Alkali-aggregate reaction	×							×	
Map cracking, crazing, scaling		×							
Joint seal damage					×				
Joint spalling	×	×							
Blowup	×								
Pumping			×	×					
Faulting				×		×			
Bumps, settlements, heaves	×					×			
Polishing						×	×		

# Table 13. Concrete pavement distresses addressed by rehabilitation and maintenance techniques.

#### DATA LIMITATIONS

The available data (including both in service and out of service test sections) present some challenges for conducting an experiment that can account for different methods of pavement preservation in a similarly robust and controlled manner to the originally designed half-factorial experiment. There are several limitations of what experiments could be conducted using the existing SPS-2 experimental sections as well as several variations that must be considered before further analysis is completed. These limitations primarily include variability across the different test sites and within in the maintenance and rehabilitation history that each section has received. These sources of variability will be discussed in the following section as well as the potential effect of these variabilities on the interpretation of proposed data, based on the selected maintenance treatments of interest.

### Intra-test site variation

During the initial design of the SPS-2 experiment, traffic was not considered as a variable to be controlled or included in the experimental design or matrix. Therefore, the anticipated traffic levels at each test site varied and were not further categorized into levels based on volume. The only site requirement for acceptance into the SPS-2 experiment was that the sections receive at least 200 kESALs annually. Over the approximate 20 years of service of the projects thus far, the traffic volumes have varied widely across the sections. Despite designing the experiment as a coupled experiment, this produces a level of difficulty when pairing the test sites and attempting to draw comparisons directly across test sections. Traffic loading as kESALs was averaged on an annual basis and presented in Table 14 below. It can be seen that traffic loading varies from as low as 248 kESALs in Delaware to as high as 3584 kESALs in Arkansas. This indicates that limiting future experiments to being within test sites may allow for drawing more valid conclusions regarding preservation treatment effectiveness.

Climatic	Climatic Conditions		State Code	State	kESALs per year
			39	Ohio	617
		Fine	19	lowa	572
Wet	Freeze		26	Michigan	1924
	Freeze		10	Delaware	248
		Coarse	5	Arkansas	3584
			55	Wisconsin	275
	No Freeze	Fine	37	North Carolina	764
	NO FIEEZE	Coarse			
			20	Kansas	719
		Fine	53	Washington	425
	Freeze	Fine	8	Colorado	383
Drav			38	North Dakota	476
Dry		Coarse	32	Nevada	739
		Fine			
	No Freeze	Coarse	6	California	1961
		Coarse	4	Arizona	1713

Table 14. Comparison of average traffic loading, in kESALs, of SPS-2 test sections.

Additionally, though without as much variance, the current age of the test sections varies by test site. Table 15 below shows the age in years for the SPS-2 test sections remaining in study. The age varies between 17 years (in California) and 24 years (in Kansas and Delaware) across all test sites.

Climatio	c Conditions	Subgrade	State	State	Age,
			Code		years
			39	Ohio	22
		Fine	19	Idaho	22
	Franza		26	Michigan	23
Wet	Freeze		10	Delaware	24
		Coarse	5	Arkansas	22
			55	Wisconsin	19
	No Freeze	Fine	37	North Carolina	23
		Coarse			
			20	Kansas	24
		Fina	53	Washington	23
	Freeze	Fine	8	Colorado	23
Draw			38	North Dakota	21
Dry		Coarse	32	Nevada	23
		Fine			
	No Freeze	Coorse	6	California	17
		Coarse	4	Arizona	23

Table 15. Comparison of age, in years, of SPS-2 test sections.

It should be noted that while the variation in age and traffic levels can be easily quantified and presented, the variation of potentially more subtle differences must also be considered, such as the exact behavior and composition of the subgrade. Despite the organization of the initial experimental matrix, the real possibility exists that a fine subgrade soil in Washington State could behave very differently than a fine subgrade soil in North Dakota, despite both being categorized by the experimental design as Dry-Freeze climates with fine subgrade soil. This becomes especially important when considering possible pavement preservation techniques given the susceptibility of soils to infiltrate cracks and joints, as well as soil susceptibility to freeze/thaw swelling can greatly impact the performance of certain maintenance activities.

#### Maintenance history variation

Potentially most limiting, many test sections have received different maintenance and rehabilitation treatments at different intervals during their lifetimes. In accordance with LTPP procedures, the activities were administered as deemed necessary by the SHA and were documented with as much detail as given by the supporting agencies. Sites remained in study unless completely overlaid or changed in a similarly fundamental capacity, and therefore, sections remaining in study have received different amounts and types of maintenance treatments at varying timing intervals. The level that previous maintenance and rehabilitation activities complicate upcoming experiments is highly dependent on the amount, timing, and variation of maintenance treatments across each of the test sections. From this information, there are several courses of action that could be taken in order to account for this variability, such as including only sections were comparably maintained (for example, a single maintenance treatment such as diamond grinding that affected all test sections equally in a single site). Additionally, preservation behaviors that are deemed extremely intrusive, such as full depth repairs, could be excluded from study.

In order to determine the extent of historical maintenance and rehabilitation treatments and their possible effect on the proposed experiment, historical maintenance and rehabilitation records were analyzed and a complete set of tables outlining the maintenance procedure and the age of the pavement during that treatment for the core experiment sections in years are given in Appendix A. These tables list the type of maintenance or rehabilitation treatments completed on each test section within a site and the age (relative to the initial pavement construction in years) of each test section during the application of that rehabilitation treatment.

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This data could additionally be used as previously described to evaluate previous maintenance treatments and eliminate sections receiving treatments deemed limiting, such as full depth repairs. However, this wealth of previous maintenance treatment data could also be used in conjunction with the current experiment to provide either more data to validate experimental observations with or to provide a means of estimating and establishing performance curves based on MEPDG prediction models.

To evaluate the present condition of the sites, the maintenance treatments were divided in terms of intrusiveness to be organized into Table 16 below. This table lists sites still in study and how many of those have or have not received any maintenance treatments. Sections are then further subdivided based on maintenance activities such that any sections receiving full depth patching, partial depth patching, or slab replacement were only counted for those maintenance actions. For example, a slab receiving both grinding and slab stabilization would be counted only under slab stabilization as that was considered the most limiting previous maintenance activity.

			Number of test sections still in study receiving this type of maintenance								
State	In study	None	Only joint or crack sealing	Only grinding	Grinding and sealing	Partial depth patching	Only skin patching	Full depth patching	Slab replacement	Only pothole patching	
AZ	12	8				4					
AR	0		8			3					
CA	12	0	9		1	2					
CO	11	6				5					
DE	12	2		1		4	1	3	1		
IA	12	5	1			4	1		1		
KS	12	0	3					7	2		
MI	0										
NV	0										
NC	6	6									
ND	12				5	6			1		
ОН	6	2		1					3		
WA	12	12									
WI	12	11								1	

# Table 16. Previous maintenance activities on test sections still in study.

#### Effect of limitations on pavement preservation techniques

These previously discussed limitations can potentially affect the direction of future analysis of SPS-2 data. The section-level variation, specifically traffic and age differences, will more greatly affect how the sections could be compared between test sections. A large difference in traffic will obviously affect any sections paired across project boundaries, such that a fair comparison may not be made. It can be seen from Table 14 that some pairings across the original half-factorial experiment are still close; for example, California and Arizona retain comparable levels of traffic (1961 and 1713 kESALs, respectively) and could be paired for a full experiment matrix. However, extrapolating results should be exercised with caution when comparing data from, for example, Michigan (1924 kESALs) with Iowa (572 kESALs) or Wisconsin (275 kESALs) with Arkansas (3584 kESALs)

Similarly, the variation in previous maintenance events can also affect the future data analysis in several ways. First, previous maintenance treatments, especially inconsistent treatments, could cause different pavement conditions between the compared sections; thus not allowing for controlled comparisons to be made. Additionally, some maintenance treatments, such as crack sealing or joint spalling repairs, will be contingent on existing distresses. Therefore, the pre-existing maintenance treatments could affect the design of the experiment, such that specific planned maintenance treatments could only exist on sections with prescribed distresses.

However, an assumption could be made that any maintenance treatments, especially limiting, aggressive treatments, were completed only to bring the condition of the entire roadway within a passable range, and therefore maintenance such as full depth repairs or slab stabilization was only completed on sections as deemed completely necessary and would create a standard road condition for the entire current existing pavement.

The previous maintenance data, which includes the extent of the maintenance activity and time, despite creating a potential discrepancy for future experimental design, does create the opportunity of a wealth of existing data that can be used for MEPDG calibration for section performance following specific maintenance treatments. This could then be used to evaluate the future performance as measured in this proposed experiment and will be discussed in more detail later.

#### **RECOMMENDED EXPERIMENTS**

#### Pairing options

Pairing sections for analysis is required for testing a preventive maintenance measure directly against a comparable section that is not improved as a control section. This will allow for further experimental design of more robust experiments. Four options for choosing paired sections across test sites will now be discussed.

#### Pairing option 1: Selecting exact experiment pairs based on experimental design

This pairing option assumes that the lane width has a negligible effect on experiment components. Logically, the lane width would only reasonably affect the performance of a dowel bar retrofit and should not affect the performance of spall repairs, partial depth repairs, or diamond grinding. Note that in Table 17 below, states with no sections left in study were completely removed for clarity and states that did not have an appropriate pair could not be included. Pairing these sections across projects does ignore the aforementioned potential variation, such as traffic levels. Sections that have been paired are outlined in bold. This method of pairing would provide 53 experimental pairs.

	Pave	ment Structu	ire		C	limatic Co	nditions ar	nd Subgrad	de
							Wet		
Drainage	Base	PCC Thickness,	Flexural strength,	Lane		Free	ze		No Freeze
Diamage	Туре	in	14-d (psi)	width	Fin	e	Соа	arse	Fine
			_ · • (po.)		ОН	IA	DE	WI	NC
			550	12	390201		100201		370201
		0	550	14		190213		550213	
		8	000	12		190214		550214	
Ne	DCAR		900	14	390202		100202		370202
No	DGAB		550	12		190215		550215	
		11	550 900	14	390203		100203		370203
		11	000	12	390204		100204		370204
			900	14		190216		550216	
			550	12	390205		100205		370205
	8	0	550	14		190217		550217	
		8	900	12		190218		550218	
Ne		900	14	390206		100206		370206	
No	LCB		550	12		190219		550219	
		11	550	14	390207		100207		370207
		11	000	12	390208		100208		370208
			900	14		190220		550220	
			550	12	390209		100209		370209
		0	550	14		190221		550221	
	8	8	000	12		190222		550222	
Vee			900	14	390210		100210		370210
Yes	PATB		550	12		190223		550223	
		11	550	14	390211		100211		370211
		11	000	12	390212		100212		370212
			900	14		190224		550224	

Table 17. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections.

	Pa	vement Struc	ture			Clima	tic Conditio	ns and Subg	grade	
		PCC	Flexural				D	ry	T	
Drainage	Base	Thickness,	strength,	Lane		Fre	eeze		No Fr	
Dramage	Туре	in	14-d	width			ine		Coa	
			(psi)		KS	ND	WA	CO	CA	AZ
			550	12	200201		530201		060201	
		8	550	14		080213		380213		04021
		0	900	12		080214		380214		04021
No	DGAB		900	14	200202		530202		060202	
INO	DGAD		550	12		080215		380215		04021
		11	550	14	200203		530203		060203	
		11	000	12	200204		530204		060204	
			900	14		080216		380216		04021
			550	12	200205		530205		060205	
		0	550	14		080217		380217		04021
		8	000	12		080218		380218		04021
NLa			900	14	200206		530206		060206	
No	LCB		550	12		080219		380219		04021
			550	14	200207		530207		060207	
		11	000	12	200208		530208		060208	
			900	14		080220		380220		04022
			550	12	200209		530209		060209	
		0	550	14		080221		380221		04022
		8		12		080222*		380222		04022
	B 4 7 5		900	14	200210		530210		060210	
Yes	ΡΑΤΒ		550	12		080223		380223		04022
	11		550	14	200211		530211		060211	
		11		12	200212		530212		060212	
		900	14		080224		380224		04022	

Table17. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections (continued)

#### Pairing option 2: Site specific ignoring base layer effects

The second pairing option identifies pairs within projects rather than across projects due to the aforementioned potential differences between traffic level, age and any other differences between sites. Because these experiments are no longer paired across sites to complete the full factorial experimental matrix, several assumptions must be made. These include:

- Unlike PCC thickness and PCC strength, the type of base layer used would not greatly affect the performance of preventive maintenance.
- The types of base layer that provide no drainage, the dense graded aggregate base and the lean concrete base were deemed sufficiently comparable to pair sites within a test section across these different base types.
- The permeable asphalt treated base, which provided drainage, was deemed sufficiently different to not be included in the pairing across base types.

The inclusion of drainage could potentially affect the type of distress or reaction to maintenance treatments and therefore should not be included in the pairings. This could be especially true for distress mechanisms which can be aggravated by inadequate subsurface drainage, such as pumping, or for increasing the level of potentially damaging incompressible materials which could infiltrate cracks or joints.

Pairs are indicated with a bold outline in Table 18. This method of pairing provides 40 experimental pairs.

	Pavemen	t Structure			C	limatic Co	nditions a	nd Subgra	de
	DCC						Wet		
Drainage	PCC Thickness,	Flexural strength,	Base	Lane		Free	ze		No Freeze
Diamage	in	14-d (psi)	Туре	width	Fin	e	Соа	arse	Fine
		- (1 7			ОН	IA	DE	WI	NC
			DGAB	12	390201		100201		370201
		550	DGAB	14		190213		550213	
		550		12	390205		100205		370205
			LCB	14		190217		550217	
	8	900		12		190214		550214	
			DGAB	14	390202		100202		370202
			1.00	12		190218		550218	
			LCB	14	390206		100206		370206
No			DGAB	12		190215		550215	
				14	390203		100203		370203
		550	1.00	12		190219		550219	
			LCB	14	390207		100207		370207
	11			12	390204		100204		370204
			DGAB	14		190216		550216	
		900	1.00	12	390208		100208		370208
			LCB	14		190220		550220	
				12	390209		100209		370209
		550		14		190221		550221	
	8	000		12		190222		550222	
Yes		900	ΡΑΤΒ	14	390210		100210		370210
res		EFO	PAIB	12		190223		550223	
	11	550		14	390211		100211		370211
		900		12	390212		100212		370212
		300		14		190224		550224	

 Table 18. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections.

	Pavem	ent Structu	re		-	Clima	tic Conditio	ns and Subg	grade	
	DCC	Flexural					D	ry		
Drainage	PCC Thickness,	strength,	Base	Lane		Fre	eeze		No F	reeze
Dramage	in	14-d	Туре	width		F	ine		Coa	arse
		(psi)			KS	ND	WA	CO	CA	AZ
			DGAB	12	200201		530201		060201	
		550	DGAB	14		080213		380213		040213
		550		12	200205		530205		060205	
	0		LCB	14		080217		380217		040217
	8			12		080214		380214		040214
		000	DGAB	14	200202		530202		060202	
		900		12		080218		380218		040218
			LCB	14	200206		530206		060206	
No			5645	12		080215		380215		040215
		550	DGAB	14	200203		530203		060203	
		550		12		080219		380219		040219
			LCB	14	200207		530207		060207	
	11			12	200204		530204		060204	
			DGAB	14		080216		380216		040216
		900		12	200208		530208		060208	
			LCB	14		080220		380220		040220
				12	200209		530209		060209	
		550	ΡΑΤΒ	14		080221		380221		040221
	ð	000	DATD	12		080222*		380222		040222
Voc	/es 8 900 11 900	900	ΡΑΤΒ	14	200210		530210		060210	
103		550	РАТВ	12		080223		380223		040223
				14	200211		530211		060211	
		900	РАТВ	12	200212		530212		060212	
		500		14		080224		380224		040224

Table 18. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections (continued)

## Pairing option 3: Selecting exact experiment pairs based on experimental design and discounting sections with limiting previous maintenance activity

Similar to the first pairing option, this method assumes that the lane width has a negligible effect on experiment components and, therefore, pairs were selected across projects. Again, it was assumed that the lane width would only reasonably affect the performance of a dowel bar retrofit and should not affect the performance of spall repairs, partial depth repairs, or diamond grinding. Note that in Table 19 below, states with no sections left in study were completely removed for clarity and states that did not have an appropriate pair could not be included. Additionally, this pairing option did not include any sections that received what was considered limiting previous maintenance activity. This included full depth repairs and slab stabilization, which were deemed to have changed the original sections too much for adequate data collection. Sections that have received these limiting maintenance activities are highlighted in gray and again, sections removed from study are highlighted in orange, in Table 19 below and are excluded from the paired sampling. Pairing these sections across projects does ignore the aforementioned potential variation, such as traffic levels. Sections that have been paired are outlined in bold. This method of pairing would provide 40 experimental pairs.

	Pave	ment Structu	ire		. (	Climatic Co	onditions a	ind Subgra	de
		500					Wet		
Drainage	Base	PCC Thickness,	Flexural strength,	Lane		Free	ze		No Freeze
Dramage	Туре	in	14-d (psi)	width	Fin	ie	Coa	arse	Fine
			- ( /		ОН	IA	DE	WI	NC
			550	12	390201		100201		370201
		8	550	14		190213		550213	
		0	900	12		190214		550214	
No	DGAB		900	14	390202		100202		370202
NO	DGAB		FF0	12		190215		550215	
		11	550	14	390203		100203		370203
		11	000	12	390204		100204		370204
			900	14		190216		550216	
			550	12	390205		100205		370205
		0	550	14		190217		550217	
	8	8	000	12		190218		550218	
			900	14	390206		100206		370206
No	LCB			12		190219		550219	
			550	14	390207		100207		370207
		11		12	390208		100208		370208
			900	14		190220		550220	
				12	390209		100209		370209
		0	550	14		190221		550221	
		8	000	12		190222		550222	
	DATO		900	14	390210		100210		370210
Yes	ΡΑΤΒ		550	12		190223		550223	
		11	550	14	390211		100211		370211
				12	390212		100212		370212
			900	14		190224		550224	

Table 19. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections.

	Pa	vement Struc	ture			Clima	tic Conditio	ns and Subg	grade	
		PCC	Flexural				D	ry		
Drainage	Base	Thickness,	strength,	Lane		Fre	eeze		No Fi	reeze
Dramage	Туре	in	14-d	width		F	ine		Coa	
			(psi)		KS	ND	WA	CO	CA	AZ
			550	12	200201		530201		060201	
		8	550	14		080213		380213		040213
		0	900	12		080214		380214		040214
No	DGAB		900	14	200202		530202		060202	
INO	DGAB		550	12		080215		380215		040215
		11	550	14	200203		530203		060203	
		11	900	12	200204		530204		060204	
			900	14		080216		380216		040216
			550	12	200205		530205		060205	
		0	550	14		080217		380217		040217
		8	000	12		080218		380218		040218
Na			900	14	200206		530206		060206	
No	LCB		550	12		080219		380219		040219
			550	14	200207		530207		060207	
		11	000	12	200208		530208		060208	
			900	14		080220		380220		040220
			550	12	200209		530209		060209	
		0	550	14		080221		380221		040222
		8	000	12		080222*		380222		040222
Maria	DATE		900	14	200210		530210		060210	
Yes	ΡΑΤΒ			12		080223		380223		040223
		550	14	200211		530211		060211		
	11		12	200212		530212		060212		
			900	14		080224		380224		040224

Table 19. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections (continued)

# Pairing option 4: Site specific ignoring base layer effects and discounting sections with limiting previous maintenance activity

Similar to pairing option 2, pairs were matched within test sites rather than across sites due to the aforementioned potential differences between traffic level and age and any potentially unquantifiable differences between sites. Because these experiments are no longer paired, several assumptions must be made. These include:

- Unlike PCC thickness and PCC strength, the type of base layer used would not greatly affect the performance of preventive maintenance.
- The types of base layer that provide no drainage, the dense graded aggregate base and the lean concrete base were deemed sufficiently comparable to pair sites within a test section across these different base types.
- The permeable asphalt treated base, which provided drainage, was deemed sufficiently different to not be included in the pairing across base types.

The inclusion of drainage could potentially affect the type of distress or reaction to maintenance treatments and therefore should not be included in the pairings.

Additionally, this pairing option did not include any sections that received what was considered to be limiting previous maintenance activity. This included full depth repairs and slab stabilization which were deemed to have changed the original sections too much for adequate data collection. Sections that have received these limiting maintenance activities are highlighted in gray in Table 20 below and are excluded from the paired sampling. Sections that have been paired are outlined in bold. This method of pairing is considered the most conservative and produced 18 experimental pairs.

	Pavemen	t Structure	•		(	Climatic Co	nditions a	nd Subgra	de
	DCC						Wet		
Drainage	PCC Thickness,	Flexural strength,	Base	Lane		Free	ze		No Freeze
Dramage	in	14-d (psi)	Туре	width	Fin	e	Соа	arse	Fine
		( /			ОН	IA	DE	WI	NC
			DGAB	12	390201		100201		370201
		550	DGAB	14		190213		550213	
		550		12	390205		100205		370205
	0		LCB	14		190217		550217	
	8			12		190214		550214	
		900	DGAB	14	390202		100202		370202
		900 -	1.00	12		190218		550218	
N			LCB	14	390206		100206		370206
No			DGAB	12		190215		550215	
				14	390203		100203		370203
		550	1.00	12		190219		550219	
			LCB	14	390207		100207		370207
	11			12	390204		100204		370204
			DGAB	14		190216		550216	
		900	1.00	12	390208		100208		370208
			LCB	14		190220		550220	
				12	390209		100209		370209
	0	550		14		190221		550221	
	8	000		12		190222		550222	
Yes		900	ΡΑΤΒ	14	390210		100210		370210
res		FEO	PAIB	12		190223		550223	
	11	550		14	390211		100211		370211
	11	900		12	390212		100212		370212
		900		14		190224		550224	

Table 20. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections.

[		Pavem	ent Structu	re		-	Clima	tic Conditio	ns and Subg	rade	•							
		DCC	Flexural					D	ry									
	Drainage	PCC Thickness,	strength,	Base	Lane		Fre	eeze		No F	reeze							
	Dramage	in	14-d	Туре	width		F	ine		Coa	arse							
			(psi)			KS	ND	WA	CO	CA	AZ							
				DGAB	12	200201		530201		060201								
			550	DGAB	14		080213		380213		040213							
			550	LCB	12	200205		530205		060205								
		8		LCB	14		080217		380217		040217							
		8			12		080214		380214		040214							
			000	DGAB	14	200202		530202		060202								
			900	1.00	12		080218		380218		040218							
				LCB	14	200206		530206		060206								
	No				12		080215		380215		040215							
				DGAB	14	200203		530203		060203								
			550	1.05	12		080219		380219		040219							
				LCB	14	200207		530207		060207								
		11			12	200204		530204		060204								
				DGAB	14		080216		380216		040216							
			900		12	200208		530208		060208								
				LCB	14		080220		380220		040220							
Ī					12	200209		530209		060209								
		0	550	ΡΑΤΒ	14		080221		380221		040221							
		Yes 8 900 11 900	000	000	000	900	900	900	900	000		12		080222*		380222		040222
	Vec		900	РАТВ	14	200210		530210		060210								
	163		550	РАТВ	12		080223		380223		040223							
					14	200211		530211		060211								
			900	РАТВ	12	200212		530212		060212								
					14		080224		380224		040224							

Table20. Summary table of status of current SPS-2 test sections and experiment design for core experiment sections (continued)

#### Experiment options for the core experiment sections

As previously discussed, several popular preventive maintenance options were selected to be included for further experimentation as follows:

- Partial depth patching (including spall repair)
- Joint sealing and/or crack sealing
- Diamond grinding
- Dowel bar retrofit

An experiment suggested for dowel bar retrofit using only the supplemental sections will be discussed in more detail later; however, the remaining four maintenance options can be used for an experiment utilizing the existing core sections. However, crack sealing, spall repair, and partial depth patching (also used for spall repair) require that specific distresses exist on the sections which will be assigned those specific methods of rehabilitation. Joint sealing and diamond grinding could be used on any section as a means of general preventive maintenance.

Only crack sealing and spall repair were distress-specific maintenance techniques and diamond grinding and joint sealing could be completed for any section, regardless of current distress. Table 21 below presents the results of the most recent distress survey that varied by site but was conducted in either 2014 or 2015. A complete list of the distresses exhibited by each core section is also given in Appendix B. The purple-shaded squares indicate sites in study that have noted spalling in the most recent distress survey and would therefore be candidates for spall repairs. In Table 22, the purple shaded squares indicate sites in study that have noted and would therefore be candidates for crack sealing. As mentioned previously, diamond grinding and joint sealing could be compared on any other section. In both tables, cells shaded in orange indicate sections that are no longer in the study.

		vement Struc	ture				-		ons and Su			0
								V	Vet			
Drainage	Base	PCC Thickness,	Flexural strength,	Lane			Fre	eeze			No Fr	eeze
Diamage	Туре	in	14-d (psi)	width		Fine			Coarse		Fine	Coarse
			1 a (psi)		OH	IA	MI	DE	AR	WI	NC	
			550	12	390201			100201			370201	
		8	220	14		190213	260213		050213	550213		
		0	900	12		190214	260214		050214	550214		
No	DGAB		900	14	390202			100202			370202	
NO	DOAD		550	12		190215	260215		050215	550215		
		11	550	14	390203			100203			370203	
		11	900	12	390204			100204			370204	
			900	14		190216	260216		050216	550216		
			550	12	390205			100205			370205	
		8	550	14		190217	260217		050217	550217		
		0	900	12		190218	260218		050218	550218		
No	LCB		900	14	390206			100206			370206	
NO	LCD		550	12		190219	260219		050219	550219		
		11	550	14	390207			100207			370207	
		11	000	12	390208			100208			370208	
			900	14		190220	260220		050220	550220		
			550	12	390209			100209			370209	
		8	550	14		190221	260221		050221	550221		
		ð	900	12		190222	260222		050222	550222		
Vac			900	14	390210			100210			370210	
Yes	ΡΑΤΒ		550	12		190223	260223		050223	550223		
			550	14	390211			100211			370211	
		11	000	12	390212			100212			370212	
			900	14		190224	260224		050224	550224		

Table 21. Summary table of current SPS-2 test sections and experiment design for core experiment sections with spalling

	Ра	vement Struc			•		-	Conditions				
			Flexural					Dry				
Drainage	Base	PCC Thickness,	strength,	Lane			Freeze				No Free	ze
Drainage	Туре	in	14-d	width		Fii	ne		Coarse	Fine	Coa	arse
			(psi)		KS	WA	CO	ND	NV		CA	AZ
			550	12	200201	530201			320201		060201	
		8	330	14			380213	080213				040213
		0	900	12			380214	080214				040214
No	DGAB		900	14	200202	530202			320202		060202	
NO	DGAD		550	12			380215	080215				040215
		11	550	14	200203	530203			320203		060203	
		11	900	12	200204	530204			320204		060204	
			900	14			380216	080216				040216
			550	12	200205	530205			320205		060205	
		0	550	14			380217	080217				040217
		8	000	12			380218	080218				040218
Na			900	14	200206	530206			320206		060206	
No	LCB		550	12			380219	080219				040219
		11	550	14	200207	530207			320207		060207	
		11	900	12	200208	530208			320208		060208	
			900	14			380220	080220				040220
			550	12	200209	530209			320209		060209	
		0	550	14			380221	080221				040221
		8	000	12			380222	080222*				040222
N	DATE		900	14	200210	530210			320210		060210	
Yes	ΡΑΤΒ			12			380223	080223				040223
		11	550	14	200211	530211			320211		060211	
		11	000	12	200212	530212			320212		060212	
			900	14			380224	080224				040224

## Table21. Summary table of current SPS-2 test sections and experiment design for core experiment sections with spalling (continued)

		avement Struc	ture						ons and Su			0
								V	/et			
Drainage	Base	PCC Thickness,	Flexural strength,	Lane			Fre	eeze			No Fr	eeze
Dialitage	Туре	in	14-d (psi)	width		Fine			Coarse		Fine	Coarse
			1 a (psi)		ОН	IA	MI	DE	AR	WI	NC	
			550	12	390201			100201			370201	
		8	550	14		190213	260213		050213	550213		
		0	900	12		190214	260214		050214	550214		
No	DGAB		900	14	390202			100202			370202	
NO	DGAD		550	12		190215	260215		050215	550215		
		11	550	14	390203			100203			370203	
		11	900	12	390204			100204			370204	
			900	14		190216	260216		050216	550216		
			550	12	390205			100205			370205	
		8	000	14		190217	260217		050217	550217		
		ŏ	900	12		190218	260218		050218	550218		
No			900	14	390206			100206			370206	
No	LCB		550	12		190219	260219		050219	550219		
		11	550	14	390207			100207			370207	
		11	000	12	390208			100208			370208	
			900	14		190220	260220		050220	550220		
			550	12	390209			100209			370209	
		0	550	14		190221	260221		050221	550221		
		8	000	12		190222	260222		050222	550222		
N.s.s	DATD		900	14	390210			100210			370210	
Yes	ΡΑΤΒ		F F 0	12		190223	260223		050223	550223		
			550 -	14	390211			100211			370211	
		11	000	12	390212			100212			370212	
			900	14		190224	260224		050224	550224		

## Table 22. Summary table of current SPS-2 test sections and experiment design for core experiment sections with cracking

	Ра	vement Struc	ture				Climatic	Conditions	and Subgra	ade		
		DCC	Flexural					Dry				
Drainage	Base	PCC Thickness,	strength,	Lane			Freeze				No Freez	ze
Diamage	Туре	in	14-d	width		Fii	ne		Coarse	Fine	Соа	arse
			(psi)		KS	WA	CO	ND	NV		CA	AZ
			550	12	200201	530201			320201		060201	
		0	550	14			380213	080213				040213
		8	000	12			380214	080214				040214
Na	DCAD		900	14	200202	530202			320202		060202	
No	DGAB			12			380215	080215				040215
		4.4	550	14	200203	530203			320203		060203	
		11	000	12	200204	530204			320204		060204	
			900	14			380216	080216				040216
				12	200205	530205			320205		060205	
		0	550	14			380217	080217				040217
		8	000	12			380218	080218				040218
Ne			900	14	200206	530206			320206		060206	
No	LCB		550	12			380219	080219				040219
		4.4	550	14	200207	530207			320207		060207	
		11	000	12	200208	530208			320208		060208	
			900	14			380220	080220				040220
			550	12	200209	530209			320209		060209	
		0	550	14			380221	080221				040221
		8	000	12			380222	080222*				040222
N a a	DATE		900	14	200210	530210			320210		060210	
Yes	ΡΑΤΒ		550	12			380223	080223				040223
		11	550	14	200211	530211			320211		060211	
		11	000	12	200212	530212			320212		060212	
			900	14			380224	080224				040224

Table 22. Summary table of current SPS-2 test sections and experiment design for core experiment sections with cracking (continued)

Considering these factors, several experimental designs can now be fully proposed.

#### Option 1: All divisions of treatment, only sections without limiting previous maintenance

In this option, sections with limiting previous maintenance, namely slab stabilization or full depth repairs, are excluded from future study and therefore either pairing option three or four could be used to pair comparable test sections (each with its own control section and excluding sections receiving limiting maintenance treatments). Each maintenance activity, including spall repair, diamond grinding, crack sealing, joint sealing, and partial depth repairs would be used within each test section. Several test sections do not have enough pairs to fulfill the criteria but the suggested experiment based on pairing option three is given in Table 23 below, and the suggested experiment design based on pairing option four is given in Table 24 below. The bold squares indicate paired test sections, and gray shaded cells indicate sections that have received a limiting maintenance treatment (excluded from experimentation) and the orange shaded cells indicate test sections that are no longer in study. The recommended compared treatment for each experimental pair in this experimental option is written in the bold square. One section should be a control section that will not receive the maintenance treatment and one section will receive the experimental maintenance treatment. The general intent was to have at least two replicate testing pairs within each similar climate block. There were some limitations based on existing distresses and some similar climate divisions had more than ten experimental pairs. In this case, additional testing replicates were based on existing distresses as applicable.

	Pav	vement Struct				÷ ;	÷ .	and Subgrade	
							Wet		
Drainage	Base	PCC Thickness,	Flexural strength,	Lane			Freeze		No Freeze
Drainage	Туре	in	14-d (psi)	width	Fine			Coarse	Fine
			. ,		ОН	IA	DE	WI	NC
			550	12	390201		100201		370201
		0	330	14	Crack and joint seal	190213		550213	
		8	000	12		190214		550214	
No	DGAB		900	14	390202		100202	Crack and joint sealing	370202
INO	DGAB		550	12	Spall repair	190215		550215	
		11	550	14	390203		100203	Spall repair	370203
		11	000	12	390204		100204	Diamond grind and joint seal	370204
			900	14		190216		550216	
			550	12	390205		100205	Crack and joint sealing	370205
		8	550	14		190217		550217	
		õ	900	12		190218		550218	
No	LCB		900	14	390206		100206	Joint seal	370206
INO	LCB		FFO	12		190219		550219	
		11	550	14	390207		100207	Spall repair	370207
		11	000	12	390208		100208	Diamond grinding	370208
			900	14		190220		550220	
			550	12	390209		100209	Diamond grind and joint seal	370209
		0	550	14		190221		550221	
		8	000	12		190222		550222	
Vaa			900	14	390210		100210	Diamond grind and joint seal	370210
Yes	ΡΑΤΒ		E E O	12	Diamond grinding	190223		550223	
		11	550	14	390211		100211	Diamond grind	370211
		11	000	12	390212		100212	Joint seal	370212
			900	14		190224		550224	

## Table 23. Proposed experimental option 1 using pairing option 3.

	Pav	ement Stru					Climatic Conditions and	-						
							Dry							
Drainage	Base	PCC Thick.,	Flexural strength,	Lane			Freeze		1	No Freeze				
Drainage	Туре	in	14-d (psi)	width			Fine			Coarse				
					KS	ND	WA	CO	CA	AZ				
			550	12	200201		530201		060201	Diamond grind and joint sealing				
		8	550	14		080213	Diamond grind and joint seal	380213		040213				
			900	12		080214	Diamond grinding	380214		040214				
No	DGAB		900	14	200202		530202		060202	Diamond grinding				
				12	Diamond grinding	080215	Crack and joint sealing	380215		040215				
		11	550	14	200203		530203		060203	Crack and joint sealing				
			900	12	200204		530204		060204	Spall repair				
			900	14		080216	Joint sealing only	380216		040216				
			550	12	200205		530205		060205	Crack and joint sealing				
		8		14		080217		380217		040217				
		ŏ		12		080218	Spall repair	380218		040218				
No	LCB						900 14		200206		530206		060206	Crack and joint sealing
			550	12		080219	Spall repair	380219		040219				
			550	14	200207		530207		060207	Spall repair				
		11	900	12	200208		530208		060208	Joint sealing only				
			900	14	Spall repair	080220	Crack and joint sealing	380220		040220				

## Table23. Proposed experimental option 1 using pairing option 3 (continued)

			Tubici		sea experimental op		g pairing option 5 (contin									
	Pav	ement Stru	cture				Climatic Conditions and	Subgrade								
							Dry									
Drainage	Base	PCC Thick.,	Flexural	Lane			Freeze		٢	No Freeze						
Drainage	Type in		strength, 14-d (psi)	width			Fine			Coarse						
			([)		KS	ND	WA	CO	CA	AZ						
				12	200209		530209		060209	Joint sealing only						
		8	550	14		080221	Diamond grinding and joint seal	380221		040221						
			000	12		080222*	Diamond grinding	380222		040222						
			900	14	200210		530210		060210	Diamond grinding						
Yes	ΡΑΤΒ			12		080223	Joint sealing only	380223		040223						
		11	550	14	200211		530211		060211	Diamond grinding and joint sealing						
			11		12		12			12	200212		530212		060212	Spall repair
			900	14	Joint and crack sealing	080224	Diamond grinding and joint sealing	380224		040224						

#### Table23. Proposed experimental option 1 using pairing option 3 (continued)

	Paver	nent Structu					Climatic Conditions a		
							Wet		
Drainage	PCC Thick.,	Flexural strength,	Base	Lane			Freeze		No Freeze
Diamage	in	14-d (psi)	Туре	width		Fine	Coa	rse	Fine
					ОН	IA	DE	WI	NC
			DCAD	12	390201		100201		370201
	550		DGAB	14		190213		550213	
		550	LCB	12	390205	Diamond grinding	100205	Spall repair	370205
			LCB	14		190217		550217	
	8			12		190214		550214	
	900	900	DGAB	14	390202	Crack and joint sealing	100202	Crack and joint sealing	370202
			LCB	12		190218	Spall repair	550218	
			LCD	14	390206		100206		370206
No				12		190215		550215	
			DGAB	14	390203	Spall repair	100203	Diamond grinding	370203
		550	LCB	12		190219	Crack and joint sealing	550219	Spall repair
				14	390207		100207		370207
	11			12	390204		100204		370204
		000	DGAB	14		190216	Diamond grinding only	550216	Diamond grinding and joint sealing
		900	900 LCB		390208	Diamond grinding and joint sealing	100208	Diamond grinding and joint sealing	370208
				14		190220		550220	

#### Table 24. Proposed experimental option 1 using pairing option 4.

	Paver	nent Structur		•			atic Conditions and Sub	-																		
							Wet																			
Drainage	PCC Thick.,	Flexural strength,	Base	Lane			Freeze		No Freeze																	
Drainage	in	14-d (psi)	Туре	width		Fine	Соа	rse	Fine																	
		, , , , , , , , , , , , , , , , , , ,			ОН	IA	DE	WI	NC																	
		550		12	390209		100209		370209																	
	8	550		14		190221		550221																		
	õ	000	900		12		190222		550222																	
Yes		900	DATD	14	390210		100210		370210																	
Yes		550	PATB	ΡΑΤΒ	PATB	PATB	PATB	ΡΑΤΒ	PATB	PATB	PATB	PATB	ΡΑΤΒ	PATB	PATB	ΡΑΤΒ	ΡΑΤΒ	ΡΑΤΒ	ΡΑΤΒ	PATB	12		190223		550223	
	11	550																		14	390211		100211		370211	
	11	000										12	390212		100212		370212									
	900			14		190224		550224																		

#### Table 25. Proposed experimental option 1 using pairing option 4 (continued).

	Paver	nent Structu					Climatic Conditions			
		Flexural					Dry			
Duringer	PCC	strength,	Base	Lane		Free	eze		No F	reeze
Drainage	Thick., in	14-d	Туре	width		Fir	e		Coa	arse
		(psi)			KS	ND	WA	CO	CA	AZ
			DGAB	12	200201		530201		060201	
			DGAB	14		080213	Spall repair	380213	Spall repair	040213
		550	550 LCB		200205		530205		060205	Diamond grinding and joint sealing
	8			14		080217		380217		040217
	0			12		080214		380214		040214
		000	DGAB		200202	Joint and crack sealing	530202	Diamond grinding	060202	Diamond grinding
		900 LCB		12		080218	Joint and crack sealing	380218	Joint and crack sealing	040218
No				14	200206		530206		060206	
NO				12		080215		380215		040215
		550	DGAB	14	200203	Spall repair	530203	Joint and crack sealing	060203	Joint and crack sealing
		550	LCB	12		080219	Diamond grinding	380219	Diamond grinding only	040219
				14	200207		530207		060207	
	11			12	200204		530204		060204	
	900		DGAB	14	Diamond grinding and joint sealing	080216	Diamond grinding and joint sealing	380216	Diamond grinding and joint sealing	040216
				12	200208		530208	Spall repair	060208	Spall repair
			LCB	14		080220		380220		040220

## Table 24. Proposed experimental option 1 using pairing option 4 (continued).

	Paver	nent Structur	e	•		Clim		ns and Subgra	de	
							Dr	У		
Drainage	PCC Thick.,	Flexural strength,	Base	Lane		Freeze	2		No	Freeze
Drainage	in	14-d (psi)	Туре	width		Fine			C	oarse
		- ( /			KS	ND	WA	CO	CA	AZ
		550	РАТВ	12	200209		530209		060209	
	8	550	PAID	14		080221		380221		040221
	ŏ	000	РАТВ	12		080222*		380222		040222
Yes		900	PAIB	14	200210		530210		060210	
res		550	РАТВ	12		080223		380223		040223
	11	550	PAID	14	200211		530211		060211	
	11	900		12	200212		530212		060212	
			ΡΑΤΒ	14		080224		380224		040224

## Table 24. Proposed experimental option 1 using pairing option 4 (continued).

#### **Option 2: All treatments types, ignoring previous maintenance**

In this testing option, sections with limiting previous maintenance, namely slab stabilization or full depth repairs, are included in the current experiment and therefore either pairing option one or two could be used to pair comparable test sections each with its own control section. Each maintenance activity including spall repair, diamond grinding, crack sealing, joint sealing, and partial depth repairs would be used within each test section, as possible. Several test sections do not have enough pairs to fulfill the criteria. The suggested experiment based on pairing option one is given in Table 26 below and the suggested experiment design based on pairing option two is given in Table 27 below. The bold squares indicate paired test sections and the orange shaded cells indicate test sections that are no longer in study. The recommended compared treatment for each experimental pair in this experimental option is written in the bold square. One section should be a control section that will not receive the maintenance treatment and one section will receive the experimental maintenance treatment. The general intent was to have at least two replicate testing pairs within each similar climate block. There were some limitations based on existing distresses and some similar climate divisions had more than ten experimental pairs. In this case, additional testing replicates were based on existing distresses as applicable.

	Pave	ment Stru	ucture		ble 26. Proposed experimental opti			and Subgrade	
							Wet		
Drainage	Base	PCC Thick.,	Flexural strength, 14-d (psi)			Free	eze		No Freeze
Drainage	Туре	in		width	Fine			Coarse	Fine
			,		ОН	IA	DE	WI	NC
			550	12	390201		100201	Joint sealing	370201
		8	550	14	Crack and joint sealing	190213		550213	
		0	900	12		190214		550214	
No	DGAB		900	14	390202		100202	Crack and joint sealing	370202
NO	DGAB		550	12	Spall repair	190215		550215	
		11	550	14	390203		100203	Spall repair	370203
		11	000	12	390204		100204	Diamond grinding and joint sealing	370204
			900	14		190216		550216	
			550	12	390205		100205	Crack and joint sealing	370205
			550	14		190217		550217	
		8		12		190218		550218	
N			900	14	390206		100206	Spall repair	370206
No	LCB		550	12	Diamond grinding and joint sealing	190219		550219	
			550	14	390207		100207	Diamond grinding	370207
		11	000	12	390208		100208	Diamond grind and joint sealing	370208
			900	14	Joint sealing	190220		550220	
			550	12	390209		100209	Spall repair	370209
			550	14		190221		550221	
		8	000	12		190222		550222	
V	DATO		900	14	390210		100210	Diamond grind and joint seal	370210
Yes	ΡΑΤΒ		550	12	Diamond grinding	190223		550223	
		11	550	14	390211		100211	Diamond grinding	370211
		11	000	12	390212		100212	Joint sealing	370212
			900	14	Spall repair	190224		550224	

## Table 26. Proposed experimental option 2 using pairing option 1.

	Davor	ment Stru	icturo	Tab	ie 25. Proposeu expe		Climatic Conditions and	,	•	
	Paver						Dry	Jongrane		
Duringen	Base	PCC	Flexural strength,	Lane			Freeze		No Freeze	
Drainage	Туре	Thick., in	14-d	width			Fine		Coarse	
			(psi)		KS	ND	WA	CO	CA	AZ
				12	200201		530201		060201	
		8	550	14	Joint and crack sealing	080213	Diamond grind and joint sealing	380213	Diamond grind and joint sealing	040213
			000	12	Spall repair	080214	Diamond grinding	380214	Diamond grinding	040214
NI-	DCAD		900	14	200202		530202		060202	
No	DGAB		550	12	Diamond grinding	080215	Crack and joint sealing	380215	Crack and joint sealing	040215
			550	14	200203		530203		060203	
		11		12	200204		530204		060204	
			900	14	Diamond grinding and joint sealing	080216	Joint sealing	380216	Spall repair	040216
			550	12	200205		530205		060205	
		0	550	14		080217	Crack and joint sealing	380217	Crack and joint sealing	040217
		8	000	12	Spall repair	080218	Spall repair	380218	Joint sealing	040218
NI-			900	14	200206		530206		060206	
No	LCB		550	12	Joint sealing	080219	Spall repair	380219	Spall repair	040219
		11	550	14	200207		530207		060207	
		11	000	12	200208		530208		060208	
			900	14	Diamond grinding	080220	Crack and joint sealing	380220	Spall repair	040220

#### Table 25. Proposed experimental option 2 using pairing option 1 (continued).

	Paver	nent Stru	cture		<u> </u>		Climatic Conditions and	•				
			Flexural				Dry					
Drainage	Base	PCC Thick.,	strength,	Lane			Freeze		No Freeze			
Dramage	Туре	in	14-d	width			Fine		Coarse			
			(psi)		KS	ND	WA	CO	CA	AZ		
				12	200209		530209		060209			
		0	550	14	Joint sealing	080221	Diamond grinding and joint sealing	380221	Joint sealing	040221		
		8	900			12	Diamond grinding and joint sealing	080222*	Diamond grinding	380222	Diamond grinding	040222
No.	DATD			14	200210		530210		060210			
Yes	ΡΑΤΒ		550	12	Diamond grinding	080223	Joint sealing	380223	Diamond grinding and joint sealing	040223		
		11		14	200211		530211		060211			
		11		12	200212		530212		060212			
				14	Joint and crack sealing	080224	Diamond grinding and joint sealing	380224	Crack and joint sealing	040224		

#### Table 25. Proposed experimental option 2 using pairing option 1 (continued).

	Paver	nent Structu					atic Conditions and Subgr	ade				
							Wet					
Drainago	PCC	Flexural	Base	Lane		Fr	eeze		No Freeze			
Drainage	Thick., in	strength, 14-d (psi)	Туре	width		Fine	Coarse		Fine			
		( )			ОН	IA	DE	WI	NC			
				12	390201		100201		370201			
		550	DGAB	14		190213		550213				
		550	1.00	12	390205	Diamond grinding	100205	Spall repair	370205			
			LCB	14		190217		550217				
	8			12		190214		550214				
		900	DGAB	14	390202	Crack and joint sealing	100202	Crack and joint sealing	370202			
		900		12		190218	Spall repair	550218				
			LCB	14	390206		100206		370206			
				12		190215		550215				
No		550	DGAB	14	390203	Spall repair	100203	Diamond grinding	370203			
		550	LCB	12	Crack and joint sealing	190219	Crack and joint sealing	550219	Spall repair			
				14	390207		100207		370207			
	11			DGAB			12	390204		100204		370204
				14		190216	Diamond grinding	550216	Diamond grinding and joint sealing			
	900	900	LCB	12	390208	Diamond grinding and joint sealing	100208	Diamond grinding and joint sealing	370208			
				14		190220		550220				

## Table 27. Proposed experimental option 2 using pairing option 2.

	Paver	nent Structur	е		Climatic Conditions and Subgrade																							
							Wet																					
Drainage	PCC Thick.,	Flexural strength,	Base	Lane		Fre	eze		No Freeze																			
Drainage	in	14-d (psi)	Туре	width		Fine	Coar	se	Fine																			
		- (1 )			ОН	IA	DE	WI	NC																			
		550		12	390209		100209		370209																			
		550		14		190221		550221																				
	8	000	PATB	РАТВ	PATB	PATB	ΡΑΤΒ	PATB	PATB	РАТВ	12		190222		550222													
Yes		900									РАТВ	РАТВ	РАТВ	ΡΑΤΒ	РАТВ	PATB	PATB	PATB	РАТВ	РАТВ	PATB	PATB	14	390210		100210		370210
res		550																					12		190223		550223	
	11	550																			14	390211		100211		370211		
	11	11 000		900	900		12	390212		100212		370212																
		900	900			900	900	900	900	900		14		190224		550224												

## Table 26. Proposed experimental option 2 using pairing option 2 (continued).

Pavement Structure					Climatic Conditions and Subgrade						
Drainage		Flexural strength, 14-d (psi)	Base Type	Lane width	Dry						
	PCC Thick.,					Fr	No Freeze				
	in					F	Coarse				
					KS	ND	WA	СО	CA	AZ	
		550	DGAB	12	200201		530201		060201		
				14	Spall repair	080213	Spall repair	380213	Spall repair	040213	
			LCB	12	200205		530205		060205	Diamond grinding and joint sealing	
				14		080217		380217		040217	
	8	900	DGAB	12		080214		380214		040214	
				14	200202	Joint and crack sealing	530202	Diamond grinding only	060202	Diamond grinding	
			LCB	12	Joint and crack sealing	080218	Joint and crack sealing	380218	Joint and crack sealing	040218	
Ne				14	200206		530206		060206		
No	11	550		12		080215		380215		040215	
			DGAB	14	200203	Spall repair	530203	Joint and crack sealing	060203	Joint and crack sealing	
			LCB	12	Diamond grinding	080219	Diamond grinding	380219	Diamond grinding	040219	
				14	200207		530207		060207		
		900		12	200204		530204		060204		
			DGAB	14	Diamond grinding and joint sealing	080216	Diamond grinding and joint sealing	380216	Diamond grinding and joint sealing	040216	
			LCB	12	200208		530208		060208		
			LCB	14		080220		380220		040220	

## Table 26. Proposed experimental option 2 using pairing option 2 (continued).

Pavement Structure					Climatic Conditions and Subgrade						
Drainage	PCC Thick., in	Flexural strength, 14-d (psi)	Base Type	-	Dry						
						Fr	No Freeze				
					Fine				Coarse		
					KS	ND	WA	СО	CA	AZ	
	8	550	PATB	12	200209		530209		060209		
Yes				14		080221		380221		040221	
		900	РАТВ	12		080222*		380222		040222	
				14	200210		530210		060210		
	11	550	РАТВ	12		080223		380223		040223	
				14	200211		530211		060211		
		900	900 PATB	12	200212		530212		060212		
				14		080224		380224		040224	

#### Table 26. Proposed experimental option 2 using pairing option 2 (continued).

## *Option 3: Isolating one type of preventive maintenance per test site ignoring all previous maintenance treatments*

In this experimental option, a single type of preventive maintenance would be selected for each test site, all previous maintenance treatments would be ignored and all test sections remaining in study would be included in this experiment. Isolating a single type of preventive maintenance for each test site would allow for robust replication and allow for a larger sample size for future statistical analyses of the performance data. Since previous limiting maintenance will have no effect on the inclusion of sites in this experiment section, either pairing option one or two would be used with its own control section. Each maintenance activity including spall repair, diamond grinding, crack sealing and joint sealing will be used across the different test sections. The suggested experiment using pairing option one is given in Table 28 below whereas the suggested experiment design based on pairing option two is given in Table 29 below. The bold squares indicate paired test sections and the orange shaded cells indicate test sections that are no longer in study. The recommended compared treatment for each experimental pair in this option is written in the bold square. One section should be a control section that will not receive the maintenance treatment and one section will receive the experimental maintenance treatment.

	Pave	ement Str		10 20.1	Climatic Conditions and Subgrade										
			Flexural strength, 14-d (psi)	Lane width	Wet										
Drainage	Base	PCC			Freeze										
Drainage	Туре	Thick., in			Fine		Fine								
			([)		ОН	IA	Wet           Coarse           IA         DE         WI           100201         Spall repair           90213         100201         Spall repair           90214         550213           90214         550214           90215         550215           90216         550215           90216         100203         Spall repair           90216         100204         Spall repair           90217         100205         Spall repair           90218         550216         100205           90219         100205         Spall repair           90219         550218         100206           90219         550218         100207           90219         100207         Spall repair           90210         100208         Spall repair           90220         100209         Spall repair           90221         100209         Spall repair           90222         550221         550221           90222         550222         550222	NC							
		0	550	12	390201		100201	Spall repair	370201						
	DGAB		550	14	Diamond grinding and joint sealing	190213		550213							
		8	900	12		190214		550214							
No				14	390202		100202	Spall repair	370202						
NO		11	550	12	Diamond grinding and joint sealing	190215		550215							
				14	390203		100203	Spall repair	370203						
			900	12	390204		100204	Spall repair	370204						
				14		190216		550216							
		8	550 900	12	390205		100205	Spall repair	370205						
	LCB			14		190217		550217							
				12		190218		550218							
No			900	14	390206		100206	Spall repair	370206						
NO			550	12	Diamond grinding and joint sealing	190219		550219							
			550	14	390207		100207	Spall repair	370207						
			900	12	390208		100208	Spall repair	370208						
											900	14	Diamond grinding and joint sealing	190220	
Vez	PATB	8	550		550	550	550	550	550	12	390209		100209	Spall repair	370209
				14		190221		550221							
			900	12		190222		550222							
				14	390210		100210	Spall repair	370210						
Yes		11	550 -	12	Diamond grinding and joint sealing	190223		550223							
				14	390211		100211	Spall repair	370211						
		11	900	12	390212		100212	Spall repair	370212						
			900	14	Diamond grinding and joint sealing	190224		550224							

#### Table 28. Proposed experimental option 3 using pairing option 1.

	Pave	ement Stru		c 27.110	posed experime		Climatic Condition				
			Flexural					)ry			
Duringen	Base	PCC	strength,	Lane		F	reeze		No Freeze		
Drainage	Туре	Thick., in	14-d	width			Fine		Coarse		
			(psi)		KS	ND	WA	CO	CA	AZ	
			550	12	200201		530201		060201		
		0	550	14	Joint sealing	080213	Diamond grinding	380213	Crack and joint sealing	040213	
		8		12	Joint sealing	080214	Diamond grinding	380214	Crack and joint sealing	040214	
Na	DCAD		900	14	200202		530202		060202		
No	DGAB		550	12	Joint sealing	080215	Diamond grinding	380215	Crack and joint sealing	040215	
			550	14	200203		530203		060203		
		11		12	200204		530204		060204		
			900	14	Joint sealing	080216	Diamond grinding	380216	Crack and joint sealing	040216	
			550	12	200205		530205		060205		
		0	550	14		080217	Diamond grinding	380217	Crack and joint sealing	040217	
		8	000	12	Joint sealing	080218	Diamond grinding	380218	Crack and joint sealing	040218	
Ne			900	14	200206		530206		060206		
No	LCB		550	12	Joint sealing	080219	Diamond grinding	380219	Crack and joint sealing	040219	
		11	550	14	200207		530207		060207		
		11	900	12	200208		530208		060208		
			900	14	Joint sealing	080220	Diamond grinding	380220	Crack and joint sealing	040220	
			550	12	200209		530209		060209		
		0	550	14	Joint sealing	080221	Diamond grinding	380221	Crack and joint sealing	040221	
		8	000	12	Joint sealing	080222*	Diamond grinding	380222	Crack and joint sealing	040222	
Vec			900	14	200210		530210		060210		
Yes	РАТВ		550	12	Joint sealing	080223	Diamond grinding	380223	Crack and joint sealing	040223	
		11	550	14	200211		530211		060211		
		11			12	200212		530212		060212	
			900	14	Joint sealing	080224	Diamond grinding	380224	Crack and joint sealing	040224	

### Table 27. Proposed experimental option 3 using pairing option 1 (continued).

	Pave	ment Structur			•	Climatic (	Conditions and S		
							Wet		
Drainago	PCC Thick.,	Flexural strength,	Base	Lane		Free	ze		No Freeze
Drainage	in	14-d (psi)	Туре	width		Fine	C	Coarse	Fine
		· · · /			ОН	IA	DE	WI	NC
				12	390201		100201		370201
			DGAB	14		190213	Spall repair	550213	
		550	LCB	12	390205	Diamond grinding and joint sealing	100205	Diamond grinding	370205
				14		190217		550217	
	8			12		190214		550214	
		900	DGAB	14	390202	Diamond grinding and joint sealing	100202	Diamond grinding	370202
			LCB	12		190218	Spall repair	550218	
			LCB	14	390206		100206		370206
No				12		190215		550215	
		550	DGAB	14	390203	Diamond grinding and joint sealing	100203	Diamond grinding	370203
		330	LCB	12	Crack and joint sealing	190219	Spall repair	550219	Joint sealing
	11			14	390207		100207		370207
				12	390204		100204		370204
			DGAB	14		190216	Spall repair	550216	Joint sealing
		900	LCB	12	390208	Diamond grinding and joint sealing	100208	Diamond grinding	370208
				14		190220		550220	

 Table 29. Proposed experimental option 3 using pairing option 2.

	Paveme	ent Structure	-	•			Conditions and		
							Wet		
Drainage	PCC	Flexural strength,	Base	Lane		Free	ze		No Freeze
Drainage	Thick., in	14-d (psi)	Туре	width		Fine	(	Coarse	Fine
		- (  )			ОН	IA	DE	WI	NC
		550		12	390209		100209		370209
	0	550		14		190221		550221	
	8	000		12		190222		550222	
Yes		900	РАТВ	14	390210		100210		370210
res		550	PAID	12		190223		550223	
	11	550		14	390211		100211		370211
	11	000		12	390212		100212		370212
		900		14		190224		550224	

 Table 28. Proposed experimental option 3 using pairing option 2 (continued).

	Paven	nent Structu					natic Conditions			
		Flexural					Dry			
Drainage	PCC Thick.,	strength,	Base	Lane		Freeze			No F	reeze
Drainage	in	14-d	Туре	width		Fine			Coa	arse
		(psi)			KS	ND	WA	CO	CA	AZ
				12	200201		530201		060201	
		550	DGAB	14	Diamond grinding and joint sealing	080213	Joint sealing	380213	Joint and crack sealing	040213
		550	LCB	12	200205		530205	Diamond grinding	060205	Spall repair
	0			14		080217		380217		040217
	8			12		080214		380214		040214
		900	DGAB	14	200202	Crack and joint sealing	530202	Diamond grinding	060202	Spall repair
		900	LCB	12	Diamond grinding and joint sealing	080218	Joint sealing	380218	Joint and crack sealing	040218
No				14	200206		530206		060206	
NO				12		080215		380215		040215
		550	DGAB	14	200203	Crack and joint sealing	530203	Diamond grinding	060203	Spall repair
		550	LCB	12	Diamond grinding and joint sealing	080219	Joint sealing	380219	Joint and crack sealing	040219
				14	200207		530207		060207	
	11			12	200204		530204		060204	
		900	DGAB	14	Diamond grinding and joint sealing	080216	Joint sealing	380216	Joint and crack sealing	040216
		900	LCB	12	200208		530208	Diamond grinding	060208	Spall repair
				14		080220		380220		040220

 Table 28. Proposed experimental option 3 using pairing option 2 (continued).

	Pavem	ent Structur	re				matic Conditions			
		Flexural					Dry			
Drainage	PCC Thick.,	strength,	Base	Lane		Freeze	5		No F	reeze
Drainage	in	14-d	Туре	width		Fine			Coa	arse
		(psi)			KS	ND	WA	CO	CA	AZ
		550	РАТВ	12	200209		530209		060209	
	0	550	PAID	14		080221		380221		040221
	8	000	DATO	12		080222*		380222		040222
Vee		900	ΡΑΤΒ	14	200210		530210		060210	
Yes		550	DATD	12		080223		380223		040223
	11	550	PATB	14	200211		530211		060211	
	11	000	DATD	12	200212		530212		060212	
		900	ΡΑΤΒ	14		080224		380224		040224

## Table 28. Proposed experimental option 3 using pairing option 2 (continued).

# Option 4: Isolating one type of preventive maintenance per test site discounting sections receiving limiting previous maintenance activities

Similar to the previous experimental option, a single type of preventive maintenance is selected for each test site; however, all previous maintenance treatments are considered and test sections that have received a limiting previous maintenance treatment would not be included in further study. Isolating a single type of preventive maintenance for each test site would allow for robust replication and allow for a larger sample size for future statistical analyses of the performance data. Since test sites with limiting maintenance will be excluded from the study, either pairing option three or four would be used with its own control section. Each maintenance activity including spall repair, diamond grinding, crack sealing and joint sealing will be used across the different test sections. The suggested experiment using pairing option three is given in Table 30 below whereas the suggested experiment design based on pairing option four is given in Table 32 below. The bold squares indicate paired test sections, the orange shaded cells indicate test sections that are no longer in study and the cells shaded gray will not be included due to the limiting maintenance treatments. The recommended compared treatment for each experimental pair in this experimental option is written in the bold square. One section should be a control section that will not receive the maintenance treatment and one section will receive the experimental maintenance treatment.

	Pav	vement Struct			Clim		ons and Subg	rade			
		D.C.C.				V	Vet				
Drainage	Base	PCC Thickness,	Flexural strength,	Lane		Freeze			No Freeze		
Drunuge	Туре	in	14-d (psi)	width	Fine		C	oarse	Fine		
					ОН	IA	DE	WI	NC		
				12	390201		100201		370201		
		8	550	14	Diamond grinding and joint sealing	190213		550213			
			000	12		190214		550214			
NIE	DCAD		900	14	390202		100202	Spall repair	370202		
No	DGAB		550	12	Diamond grinding and joint sealing	190215		550215			
		11		14	390203		100203	Spall repair	370203		
			000	12	390204		100204	Spall repair	370204		
			900	14		190216		550216			
			550	12	390205		100205		370205		
			550	14		190217		550217			
		8	000	12		190218		550218			
	1.00		900	14	390206		100206	Spall repair	370206		
No	LCB		550	12		190219		550219			
		11	550	14	390207		100207	Spall repair	370207		
			11	11	11	000	12	390208		100208	Spall repair
			900	14		190220		550220			

 Table 30. Proposed experimental option 4 using pairing option 3.

	Pave	ment Structur	-	•	Clim		ions and Sul		
						,	Wet		
		PCC	Flexural	Lane		Freeze			No Freeze
Drainage	Base Type	Thickness, in	strength, 14-d (psi)	width	Fine		(	Coarse	Fine
					ОН	IA	DE	WI	NC
			550	12	390209		100209		370209
		8	550	14		190221		550221	
		ŏ	000	12		190222		550222	
			900	14	390210		100210		370210
Yes	ΡΑΤΒ		550	12	Diamond grinding and joint sealing	190223		550223	
		11		14	390211		100211	Spall repair	370211
				12	390212		100212	Spall repair	370212
			900	14		190224		550224	

 Table 31. Proposed experimental option 4 using pairing option 3 (continued).

	Pav	ement Str					Climatic Condi			
								Dry	0	
<b>.</b> .	Base	PCC	Flexural	Lane			Freeze		No Freeze	
Drainage	Туре	Thick., in	strength, 14-d (psi)	width			Fine		Coarse	
			« (po.)		KS	ND	WA	CO	CA	AZ
			550	12	200201		530201		060201	
		0	550	14		080213	Diamond grinding	380213	Crack and joint sealing	040213
		8		12		080214	Diamond grinding	380214		040214
N	DCAD		900	14	200202		530202		060202	
No	DGAB		550	12	Joint sealing	080215	Diamond grinding	380215	Crack and joint sealing	040215
			550	14	200203		530203		060203	
		11	000	12	200204		530204		060204	
			900	14		080216	Diamond grinding	380216	Crack and joint sealing	040216
			550	12	200205		530205		060205	
		8	550	14		080217		380217	Crack and joint sealing	040217
		õ	000	12		080218	Diamond grinding	380218	Crack and joint sealing	040218
No			900	14	200206		530206		060206	
No	LCB		550	12		080219	Diamond grinding	380219	Crack and joint sealing	040219
		11	550	14	200207		530207		060207	
		11	900	12	200208		530208		060208	
			900	14	Joint sealing	080220	Diamond grinding	380220	Crack and joint sealing	040220
			550	12	200209		530209		060209	
		0	550	14		080221	Diamond grinding	380221	Crack and joint sealing	040221
		8	000	12		080222*	Diamond grinding	380222	Crack and joint sealing	040222
Vac	DATD		900	14	200210		530210		060210	
Yes	ΡΑΤΒ		550	12		080223	Diamond grinding	380223	Crack and joint sealing	040223
		11	550	14	200211		530211		060211	
			900	12	200212		530212		060212	
			900	14	Joint sealing	080224	Diamond grinding	380224	Crack and joint sealing	040224

### Table 29. Proposed experimental option 4 using pairing option 3 (continued).

	Paver	nent Structu				a experimental op	• •	nditions and Subgrade		
		Flexural						Wet		
<b>.</b> .	PCC	strength,	Base	Lane	-		Freeze	e		No Freeze
Drainage	Thick., in	14-d	Туре	width		Fine		Coarse		Fine
		(psi)			ОН	IA	DE	AR	WI	NC
			DCAD	12	390201		100201			370201
			DGAB	14		190213		050213	550213	
		550	LCB	12	390205	Diamond grinding and joint sealing	100205		Joint sealing	370205
	8			14		190217		050217	550217	
	0			12		190214		050214	550214	
		900	DGAB	14	390202	Diamond grinding and joint sealing	100202	Diamond grinding	Joint sealing	370202
			LCB	12		190218	Spall repair	050218	550218	
No			LCB	14	390206		100206			370206
				12		190215		050215	550215	
		550	DGAB	14	390203	Diamond grinding and joint sealing	100203	Diamond grinding	Joint sealing	370203
			LCB	12		190219	Spall repair	050219	550219	Spall repair
			LCD	14	390207		100207			370207
	11		DCAD	12	390204		100204			370204
			DGAB	14		190216	Spall repair	050216	550216	Spall repair
		900	LCB	12	390208	Diamond grinding and joint sealing	100208	Diamond grinding	Joint sealing	370208
				14		190220		050220	550220	

## Table 32. Proposed experimental option 4 using pairing option 4.

	Pavem	ent Structur		•		•		nditions and Subgrade		
		Flexural						Wet		
Drainage	PCC Thick.,	strength,	Base	Lane			Freeze	e		No Freeze
Drainage	in	14-d	Туре	width		Fine		Coarse		Fine
		(psi)			ОН	IA	DE	AR	WI	NC
		550		12	390209		100209			370209
	8	550		14		190221		050221	550221	
	8	000		12		190222		050222	550222	
Yes		900	ΡΑΤΒ	14	390210		100210			370210
res		550	PAID	12		190223		050223	550223	
	11	550		14	390211		100211			370211
	11	000		12	390212		100212			370212
		900		14		190224		050224	550224	

## Table 30. Proposed experimental option 4 using pairing option 4 (continued).

	Pavem	ent Structu			roposed experime		Climatic Conditio	-	-	
		Flexural					D	ry		
Drainaga	PCC	strength,	Base	Lane		Freeze	5		No	Freeze
Drainage	Thick., in	14-d	Туре	width		Fine			Co	oarse
		(psi)			KS	ND	WA	CO	CA	AZ
				12	200201		530201		060201	
			DGAB	14		080213	Spall repair	380213	Spall repair	040213
		550	LCB	12	200205		530205		060205	Diamond grinding and joint sealing
				14		080217		380217		040217
	8			12		080214		380214		040214
		900	DGAB	14	200202	Joint and crack sealing	530202	Diamond grinding	060202	Diamond grinding
		900	LCB	12		080218	Joint and crack sealing	380218	Joint and crack sealing	040218
				14	200206		530206		060206	
No				12		080215		380215		040215
NO		550	DGAB	14	200203	Spall repair	530203	Joint and crack sealing	060203	Joint and crack sealing
			LCB	12		080219	Diamond grinding	380219	Diamond grinding	040219
				14	200207		530207		060207	
	11			12	200204		530204		060204	
		900	DGAB	14	Diamond grinding and joint sealing	080216	Diamond grinding and joint sealing	380216	Diamond grinding and joint sealing	040216
			LCB	12	200208		530208	Spall repair	060208	Spall repair
				14		080220		380220		040220

## Table 30. Proposed experimental option 4 using pairing option 4 (continued).

	Paveme	ent Structur	е				Climatic Condit	ions and Subgra	ade														
		Flexural						Dry															
Drainaga	PCC Thick.,	strength,	Base	Lane		Freeze	2		No	Freeze													
Drainage	in	14-d	Туре	width		Fine			Coarse														
		(psi)			KS	ND	WA	CO	CA	AZ													
		550	DATD	12	200209		530209		060209														
		550	PATB	14		080221		380221		040221													
	8		900	DATD	12		080222		380222		040222												
N		900	PATB	14	200210		530210		060210														
Yes		550	550	550 PATE	550	550	550	550	550	550	550	550	550	550	550	DATD	12		080223		380223		040223
	11				PATB	ΡΑΤΒ	ΡΑΤΒ	ΡΑΤΒ	PATB	РАТВ	ΡΑΤΒ	ΡΑΤΒ	ΡΑΤΒ	ΡΑΤΒ	РАТВ	РАТВ	14	200211		530211		060211	
	11 -	900 PATB						12	200212		530212		060212										
			900	900	900	900	900	900	PAIB	14		080224		380224		040224							

#### Table 30. Proposed experimental option 4 using pairing option 4 (continued).

#### Experiment possibilities for the supplemental experiment sections

As previously discussed, the supplemental sections remaining in study would provide an ideal opportunity to investigate the effects of a dowel bar retrofit as a preventive maintenance strategy for the currently undoweled sections. These supplemental sections that are currently doweled could be further divided to isolate the effects of diamond grinding only on load transfer performance, which could also be compared then to the undoweled sections. All test sections in the core experiment were required to contain specified dowel bars. Therefore, the supplemental sections that were not constructed without dowel bars would provide an excellent opportunity to investigate the impact on load transfer for undoweled sections.

The inherent difficulty of designing an experiment with the supplemental sections from the SPS-2 experiment is the lack of consistency between the test sections. As mentioned previously, the original intent of the supplemental experiment of the SPS-2 sections was to provide a robust secondary experiment to investigate dowel bar effects. However, states were more receptive to the flexibility of designing their own experiments based on issues and interest specific to that state. Many states opted to construct only a control section that was constructed with the state standard paving mix design and pavement structure. Therefore, the ability to use the sites as replicate experiments decreases substantially.

This dowel bar based experiment for the supplemental sections will have two categories of experiment: either doweled sections will be diamond ground and compared to a comparable controlled section, or undoweled sections will undergo a dowel bar retrofit and also be compared to comparable control section. Ideally, load transfer efficiency performance could be compared between both experiments. Unfortunately, states with only a control section were eliminated from this experiment because it would require at least pairs in order to match each treatment section with a control section. States with more than a single control sections. To keep cohesion across sections and to allow for broader trend interpretation, the type of dowel bar used in retrofitting should be kept consistent across all test sections while the size may vary due to variances in pavement thickness.

#### Arizona

The supplemental sections in Arizona were divided into several sub-experiments as shown in Table 33 below. Due to the variety of the inclusion of dowel bars, these test sites are ideally suited for a dowel bar retrofit experiment. The test sections without dowel bars are paired into a control section and a section that will receive dowel bars. Of the three sections with dowel bars, one will remain as a control while two will receive diamond grinding. These sections do have different PCC thicknesses and while PCC thickness should have a negligible effect on the performance of diamond grinding, it could possibly have an effect on the PCC joint faulting. However, caution should be exercised when comparing these specimens. The recommended experiment pairings are then given in Table 34. The asphalt sections were not included in this experiment.

SHRP ID	Sub- experiment	Lane width, ft	Base type	PCC thickness, in	Dowels			
040262		14	DGAB	8	No			
040263	1	14	PBTB	8	No			
040264	1	12	PBTB	11	No			
040265		12	DGAB	11	No			
040266		14	BTB	12.5	Yes			
040267	2	14	BTB	11	Yes			
040268		14	BTB	8	Yes			
040260	2	(intentional	ly identical a	and on either end of th	ne project;			
040261	3	asphalt surface)						

Table 33. Supplemental sections constructed for the Arizona SPS-2 experiment.

SHRP ID	Lane width, ft	Base type	PCC thickness, in	Dowels	New experiment type	Treatment
040262	14	DGAB	8	No	Control	None
040263	14	PBTB	8	No	Testing	Dowels
040264	12	РВТВ	11	No	Control	None
040265	12	DGAB	11	No	Testing	Dowels
040266	14	BTB	12.5	Yes	Testing	Diamond grinding
040267	14	BTB	11	Yes	Control	None
040268	14	втв	8	Yes	Testing	Diamond grinding

#### Delaware

In Delaware, the original supplemental test sections were kept consistent with the exception of the type of dowel bar used as seen in Table 35 below. Unfortunately, these sections could not be compared directly due to the different dowel bars used which would not create a replicate experiment.

SHRP ID	Lane width, ft	Compressive strength, psi	DGAB thickness, in	PCC thickness, in	Dowel type
100259	12	3000	8	10	steel
100260	12	3000	8	10	plastic

Table 35. Supplemental sections constructed for the Delaware SPS-2 experiment.

#### North Dakota

The supplemental sections of North Dakota create some level of difficulty in finding exactly matching pairs for a robust experiment. Some assumptions will have to be made. Most significantly, the joint spacing is variable for most sections due to the nature of the experiment which could affect the performance of a dowel bar retrofit. However, the same procedure is still used to create testing pairs for the recommended experiment. The experimental sections and recommendations for testing are given in Table 36 below. Due to the variability, especially of joint spacing and the pavement width, not all sections could be included in the final recommended experiment.

Table 36. Supplemental sections constructed for the North Dakota SPS-2 experiment.

SHRP ID	PCC Pavement Thickness (in)	Strength	PCC Pavement width, ft	Base Type	Dowels	Joint spacing	New Experiment type	Treatment
380260	11	***	38	DGAB	Yes	15 ft	Testing	Diamond grinding
380259*	10	***	24	8" salvaged layer (unknown material)	Yes	15 ft	Control	None
380261	11	550	24	DGAB	No	Variable **	Control	None
380262	11	550	28	LCB	No	Variable **	Testing	Dowels
380263	11	550	24	PASB	No	Variable **	Testing	Dowels
380264	11	***	38	PASB	No	15 ft		

\* indicates state control section

\*\* "variable" indicates joint spacing varying from 12, 15, 13, and 14 ft

\*\*\* considered Class AE concrete as per NDDOT specifications

Ohio

The Ohio test sections can only be paired based on mix design and base thickness; however, the effects of base type would have to be ignored for the sake of pairing sections for a dowel bar retrofit experiment. The supplementary sections in Ohio are given in Table 37 below. The detail of the specific mix designs A and B were given previously in Table 8. All test sections in Ohio were doweled so in this experimental case in Ohio, only the effect of diamond grinding can be tested for sections, rather than introducing dowel bars into an undoweled section. Section 390264 would not be included in the experiment.

SHRP ID	PCC thickness	AB, in	Base type	Base thickness, in	Mix design	New Experiment Type	Treatment
390259	11	6		0	А	Control	None
390263	11	6		0	А	Testing	Diamond grinding
390261	11	4	CTFDB	4	А	Control	None
390265	11	4	РАТВ	4	А	Testing	Diamond grinding
390260	11	4	PATB	4	В	Control	None
390262	11	4	CTFDB	4	В	Testing	Diamond grinding
390264	11	6		0	В		

Table 37. Supplemental sections constructed for the Ohio SPS-2 experiment.

#### Wisconsin

The Wisconsin supplemental test sections, like Ohio, were all doweled sections and therefore only a comparison of the effects of diamond grinding can be compared. The experimental sections, paired as recommended for this experiment, are given in Table 38 below. Not all sections were sufficiently replicable, such as section 550262, which was the only section with 900 psi strength concrete.

Table 38. Supplemental sections constructed for the Wisconsin SPS-2 experiment.

SHRP ID	Lane width, ft	Subbase thick., in	Rock base thick., in	Embankment fill thick., in	DGAB thick., in	CSOGB thick., in	PCC thick., in	PCC strength, psi	New experiment section
550259*	14			24	6		11	550	Control
550260*	14			24	6		11	550	Testing
550261	12			24	4	4	8	550	
550262	12	10	3		6		8	900	

550263	14		24	6		10	550	Control
550264	14		24	6		11	550	Testing
550265	14	10		6	4	11	550	
550266	14		24	6		11		

(\*) indicates control section

#### Data and suggested analysis

These possible experiments allow for several data analysis options. The nature of the experimental framework: namely, including a single control section matched with each test section, should allow for more robust statistical analysis than prior experiments. This would allow for data comparisons across different treatment methods to be normalized to a control and this normalized value could be used to compare treatments across the different treatment methods.

The outlined experiments were presented such that many replicates exist across different sections, although the ultimate number of sections included varies based on selected pairing option, which will have an effect on the robustness of possible statistical analysis. While a higher number of sections could potentially produce a more rigorous statistical comparison due to the higher degree of freedom, the more conservative pairing options could allow for more distinct trends to emerge. For example, despite pairing options one and two producing many more possible experimental pairings, there could be effects from ignoring previous maintenance treatments that could possibly skew observed trends which might be eliminated if using pairing options three or four.

The experimental options two and four that recommended multiple replications of the same experiment across a single project provide the opportunity for two outlets of analysis. Either the experiment could remain as recommended which would allow for more single replicates for robust statistical analysis, or the replication could allow for the opportunity to incorporate timing effects into the experiment. The options for including timing considerations could vary widely but should be held consistent across the experiments for the sake of later data analysis. A sample of testing timing across four experimental pairs for a treatment type is given in Table 39 below where time X, Y, and Z fall chronologically.

	<b>—</b> : 0	<b>— — — — —</b>	<del>-</del> ·	
Test section	Time 0	Time X	Time Y	Time Z
Test section 1	Receives initial maintenance treatment	No treatment	No treatment	No treatment
Test section 2	Receives initial maintenance treatment	Receives diamond grinding and joint sealing as a proactive follow up treatment	No treatment	No treatment
Test section 3	Receives initial maintenance treatment	Receives diamond grinding and joint sealing as a proactive follow up treatment	Receives diamond grinding and joint sealing as a proactive follow up treatment	No treatment
Test section 4	Receives initial maintenance treatment	Receives diamond grinding and joint sealing as a proactive follow up treatment	Receives diamond grinding and joint sealing as a proactive follow up treatment	Receives diamond grinding and joint sealing as a proactive follow up treatment

Table 39. Suggested time-based maintenance experiment for experiment options 2 or 4.

A limitation of the experiment design that affects the possible analysis is certainly the need to pair each site with a comparable section as a control section. As mentioned before, for many sites, this requires some assumptions regarding which pavement structure or mix design variables have a negligible effect on the tested pavement preservation techniques. The possibility exists that pavement performance could be adequately simulated by the AASHTOWARE PavementME such that the simulated performance could be used as a control section, thus effectively doubling the amount of experimental sections that would be required to complete the experiment.

In order to use the results of AASHTOWARE PavementME predictions to replace the planned control sections, the historic data taken over the life of the pavement to date would be used to establish the current performance curve with respect to time. At this point, the predicted performance curves could be constructed from the original constructed data and the predicted, based on PavementME, could be compared with the actual. If the two are comparable, it could be reasonably assumed that PavementME could be used to simulate control sections from these existing sections before receiving any treatment. Then, all of the eligible test sections could be used for experimental treatment testing without requiring a control section for each experimental pair. However, this calibration could only be

completed if it shown that the predicted and actual performance curves from PavementME are sufficiently close. It would be recommended that if the performance curves align such that the control section can be removed from the study, thus effectively doubling the amount of testing sections, that this course of action be taken.

The calibration of the PavementME performance curves from the historical data available for the SPS-2 experiment would provide the opportunity to calibrate the performance curves considering maintenance treatments and timing received by the test sections. This would provide very valuable insight into the effect of maintenance treatments and timing and how they relate to the calculated performance curves from PavementME.

#### CONCLUSION

The Long Term Pavement Performance (LTPP) SPS-2 experiment - strategic study of structural factors for rigid pavements - is the most comprehensive on-going concrete pavement research effort undertaken since the AASHO Road Test. Spanning fourteen states, the study begin in 1992 and eleven of the original fourteen sections remain in service with current years of service ranging between 14 and 22 years as of 2015. Given this unparalleled resource of well documented and monitored aged concrete pavements, the sections currently in study provide the ideal opportunity to develop a second experiment to compare the effectiveness of concrete pavement preservation strategies to extend pavement service life. Recognizing the opportunity, this pooled fund study (TPF-5(291)) was initiated to develop and implement a continuation experiment focused on pavement preservation. As a precursor to the full experiment, the evaluation and assessment of the existing SPS-2 sections with current data limitations and availability must be analyzed and discussed in order to proceed with the development of a robust experimental plan.

This report outlines the current availability of LTPP data, including sections remaining in study, and of those, which have received maintenance or rehabilitation treatments that may limit the options of inclusion in further study. Pavement preservation techniques are evaluated and discussed, including limitations that should be considered due to site specific factors or test section history. The available test sections were considered with the respective potential limitations based on the investigated pavement preservation methods. Multiple options of pairing the test sections for evaluating a control and a testing section were presented and several potential experiments were identified to investigate the performance of spall repairs, joint sealing, diamond grinding, crack sealing, and the combination maintenance of joint sealing and diamond grinding.

Additionally, an experiment to investigate the effectiveness of dowel bar retrofit is presented that utilizes the original supplemental sections, some of which were originally undoweled. This could allow for comparing the variation in load transfer efficiency and difference in faulting between a previously undoweled section receiving a dowel bar retrofit and sections with dowels that will receive diamond grinding for maintenance.

The research team believes the proposed experiment could be expanded using the wealth of existing pavement performance data from the SPS-2 experiment to utilize the predicted performance curves produced from AASHTOWARE PavementME as the "control" sections. If successful, this would eliminate paired sections and allow for doubling of the experimental sections that could be used.

Additional work will be necessary to establish the validity of this approach and revise the experimental plan.

Each pairing and experimental options, with complete discussions of analytical strengths and weaknesses of each, is given in much more detail in the accompanying report. This supplement serves to summarize the findings presented and to seek input from the panel for direction of the project. A pairing and experimental option could be chosen for further progress, or the experiment could be expanded to investigate the feasibility of utilizing AASTHOWARE PavementME curves as control sections.

#### REFERENCES

Wilde, W.J., L. Thompson, and T. J. Wood. 2014. Cost-Effective Pavement Preservation Solutions for the Real World. MN/RC 2014-33. Minnesota Local Road Research Board, Minnesota Department of Transportation

Hall, K.T., C. E. Correa, S.H. Carpenter, R.P. Elliot. 2001. Rehabilitation Strategies for Highway Pavements. NCHRP Web document 35 (Project C1-38): Contractor's Final Report.

PCA R&D Serial No. 2155. Portland Cement Association, Skokie, IL. Concrete Information: Concrete Slab Surface Defects: Causes, Prevention, Repair.

Smith, K. and D. Harrington with L. Pierce, P. Ram, and K. Smith. 2014. Concrete Pavement Preservation Guide, Second Edition. FHWA-HIF-14-014. National Concrete Pavement Technology Center sponsored by the Federal Highway Administration Appendix A. Tables of Test Section Rehabilitation and Maintenance

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
					040213		
	DGAB				Partial depth patching, not joint	16.6	
No					Partial depth patching, joints	16.6	
					040217	1	
	LCB		550	14	Partial depth patching, not joint	16.6	
			330	14	Partial depth patching, joints	16.6	
		8			040221		
Yes	Yes PATB				Partial depth patching, joints	14.5	
163	TAID				Partial depth patching, not joint	16.6	
						Partial depth patching, joints	16.6
	DGAB				040214		
No	LCB		900		040218		
	LCD		900		Partial depth patching, joints	14.5	
Yes	PATB			12	040222		
No	DGAB				040215		
No	LCB		550		040219		
Yes	PATB	11			040223		
No	DGAB	11	900		040216		
No	LCB			14	040220		
Yes	PATB				040224		

Table A-1. Maintenance and Rehabilitation for sites in Arizona.

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
					050213		
					Lane-Shoulder, Longitudinal joint sealing	3.4	
	DGAB				Crack sealing	9.3	
					Partial depth patching, not joint	10.2	
No					Partial depth patching, not joint	13.1	
NO					050217		
			550	14	Lane-Shoulder, Longitudinal joint sealing	3.4	
	LCB		550	14	Crack sealing	9.3	
					Partial depth patching, joints	9.3	
					Partial depth patching, joints	12.8	
					050221		
Yes	РАТВ	8			Lane-Shoulder, Longitudinal joint sealing	3.4	
103	IAID				Transverse joint sealing	9.3	
					Lane-Shoulder, Longitudinal joint sealing	9.3	
	DGAB				050214		
	00/10				Lane-Shoulder, Longitudinal joint sealing	3.4	
					050218	1	
No						Lane-Shoulder, Longitudinal joint sealing	3.4
	LCB		900		Crack sealing	9.3	
						Transverse joint sealing	9.3
					Partial depth patching, joints	12.8	
Yes	РАТВ			12	050222	I	
					Lane-Shoulder, Longitudinal joint sealing	3.4	
	DGAB				050215	I	
No					Lane-Shoulder, Longitudinal joint sealing	3.4	
	LCB		550		050219	1	
					Lane-Shoulder, Longitudinal joint sealing	3.4	
Yes	РАТВ				050223	1	
					Lane-Shoulder, Longitudinal joint sealing	3.4	
		11			050216	1	
	DGAB				Lane-Shoulder, Longitudinal joint sealing	3.4	
	No	-			Transverse joint sealing	9.3	
No			900	14	Lane-Shoulder, Longitudinal joint sealing	9.3	
					050220		
	LCB				Lane-Shoulder, Longitudinal joint sealing	3.4	
					Transverse joint sealing	9.3	
		Shaded cells	<u> </u>	<u> </u>	Lane-Shoulder, Longitudinal joint sealing	3.4	

## • Sections with nothing listed have not received any maintenance or rehabilitation Table A-2. Maintenance and Rehabilitation for sites in Arkansas (continued).

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	
					050224	
					Partial depth patching, joints	2.8
Yes	PATB	11	900	14	Lane-Shoulder, Longitudinal joint sealing	3.4
					050224 Partial depth patching, joints 14 Lane-Shoulder, Longitudinal joint sealing Partial depth patching, joints	7.8
					Transverse joint sealing	9.3

Notes: • Shaded cells are no longer in study

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
					060201	
					Lane-Shoulder, Longitudinal joint sealing	2.3
	DGAB				Partial depth patching, not joint	8.0
No					Partial depth patching, not joint	13.3
			550	12	060205	I
	LCB				Lane-Shoulder, Longitudinal joint sealing	2.3
					060209	I
Yes	РАТВ				Lane-Shoulder, Longitudinal joint sealing	2.3
					Partial depth patching, joints	13.3
		8			060202	
	DGAB			Lane-Shoulder, Longitudinal joint sea	Lane-Shoulder, Longitudinal joint sealing	5.3
No					060206	
	LCB				Transverse joint sealing	8.0
			900		Lane-Shoulder, Longitudinal joint sealing	8.0
	РАТВ				060210	
					Lane-Shoulder, Longitudinal joint sealing	5.3
Yes					Transverse joint sealing	8.0
					Lane-Shoulder, Longitudinal joint sealing	8.0
					060203	
				14	Transverse joint sealing	2.3
	DGAB				Lane-Shoulder, Longitudinal joint sealing	2.3
					Grinding/Milling surface	5.3
No					Grinding/Milling surface	9.0
		1	550		060207	
	LCB				Transverse joint sealing	8.0
					Lane-Shoulder, Longitudinal joint sealing	8.0
					060211	
Yes	РАТВ	11			Transverse joint sealing	2.3
					Lane-Shoulder, Longitudinal joint sealing	2.3
					060204	
					Lane-Shoulder, Longitudinal joint sealing	5.3
	DGAB				Transverse joint sealing	8.0
N			000	12	Lane-Shoulder, Longitudinal joint sealing	8.0
No		1	900	12	060208	ł
	1.05				Lane-Shoulder, Longitudinal joint sealing	2.3
	LCB				Lane-Shoulder, Longitudinal joint sealing	5.3
					Lane-Shoulder, Longitudinal joint sealing	8.0
<u>No</u>	otes: •	Shaded cells	are no lon	ger in st		

Table A-3. Maintenance and Rehabilitation for sites in California.

## • Sections with nothing listed have not received any maintenance or rehabilitation Table A-3. Maintenance and Rehabilitation for sites in California (continued).

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
Yes	РАТВ	11	900	12	060212	
	FAID	11	900	12	Lane-Shoulder, Longitudinal joint sealing	5.3

Notes: • Shaded cells are no longer in study

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
	DGAB				080213		
No					080217		
NO	LCB		550	14	Partial depth patching, joints	12.4	
					Partial depth patching, not joint	17.5	
Yes	ΡΑΤΒ				080221		
	DGAB				080214		
No		8			080218		
NO	LCB				Partial depth patching, joints	12.4	
			000		Partial depth patching, joints	15.4	
	РАТВ		900		080222		
Yes		В			Partial depth patching, joints	6.4	
165	FAID			12	Partial depth patching, joints	11.4	
					Partial depth patching, joints	12.4	
					080215		
No	DGAB				Partial depth patching, joints	11.4	
NO			550		Partial depth patching, joints	13.4	
	LCB				080219		
Yes	PATB	11			080223		
		11			080216		
	DGAB				Partial depth patching, not joint	12.4	
No			900	14	080220		
	LCB				80224		
	PATB				Partial depth patching, joints	11.3	

Table A-4.Maintenance and Rehabilitation for sites in Colorado.

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
			- (1 /		100201		
	DGAB				Full Depth joint repair patching	4.3	
					Grinding/Milling surface	8.4	
					100205		
					Full Depth joint repair patching	4.3	
No					PCC Slab replacement	4.3	
	LCB		550	12	Grinding/Milling surface	8.4	
	LCD		550	12	Transverse joint sealing	15.4	
					Lane-Shoulder, Longitudinal joint sealing	15.4	
					Full Depth joint repair patching	20.2	
		8			Skin patching	22.4	
		C C			100209	1	
Yes	ΡΑΤΒ				Full Depth joint repair patching	4.3	
					Grinding/Milling surface	8.4	
No	DGAB				100202		
	LCB		900		100206		
	РАТВ				100210	1	
					Full Depth joint repair patching	4.3	
Yes					Grinding/Milling surface	8.4	
					Transverse joint sealing	10.4	
					Lane-Shoulder, Longitudinal joint sealing	10.4	
					Skin patching	22.4	
	DGAB				100203		
					Skin patching	22.4	
				14	100207		
					Lane-Shoulder, Longitudinal joint sealing	14.4	
No					Crack sealing	16.4	
	LCB				Other	16.4	
		11	550		Crack sealing	19.2	
					Partial depth patching, not joint	19.2	
					Patch potholes, by hand	19.2	
					Skin patching 100211	22.4	
					Lane-Shoulder, Longitudinal joint sealing	14.4	
Yes	PATB					14.4	
					Partial depth patching, not joint	19.2	
N.	otes: •	Shaded cell			Skin patching	22.4	

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
					100204	_
	DGAB				Grinding/Milling surface	8.4 19.4 22.4 8.4 14.4 19.2 20.3 20.3 20.3 22.4
	DOAD				Partial depth patching, joints	19.4
					Skin patching	22.4
					100208	
No					Grinding/Milling surface	8.4
		11	900	12	Lane-Shoulder, Longitudinal joint sealing	19.4 22.4 8.4 14.4 19.2 20.3 20.3
	LCB	11	900	12	Patch potholes, by hand	19.2
					Partial depth patching, not joint	20.3
					Partial depth patching, joints	20.3
					Skin patching	22.4
Yes					100212	
	PATB				Grinding/Milling surface	8.4
					Skin patching	22.4

## Table A-5. Maintenance and Rehabilitation for sites in Delaware (continued)

Notes: • Shaded cells are no longer in study

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age			
	DGAB				190213	•			
					190217				
					PCC Slab replacement	11.4			
					Transverse joint sealing	12.4			
No					Lane-Shoulder, Longitudinal joint sealing	12.4			
NO	LCB		550	14	Full Depth patching, not joint	12.4			
					PCC Slab replacement	12.4			
					PCC Slab replacement	19.4			
					Partial depth patching, joints	19.4			
		8			Skin patching	20.4			
Yes	PATB	0			190221				
	DGAB				190214				
	DUAD				Crack sealing	19.4			
No	LCB				190218				
NO					Partial depth patching, not joint	19.4			
	LCD		900	12	Partial depth patching, joints	19.4 19.4 20.4			
		РАТВ			Skin patching	20.4			
	РАТВ				190222				
Yes					Partial depth patching, joints	19.4			
					Skin patching	20.4			
No	DGAB				190215				
NO	LCB		550	12	Skin patching         20.4           190215         190219				
Yes	PATB				190223				
	DGAB				190216				
	00/10				Skin patching	20.4			
No		11			190220				
	LCB				Partial depth patching, joints	19.4			
			900	14	Skin patching	20.4			
					190224				
Yes	РАТВ				Partial depth patching, joints	19.4			
105					Partial depth patching, not joint	19.4			
					Skin patching	20.4			

Table A-6.Maintenance and Rehabilitation for sites in Iowa.

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
					200201	
					Partial depth patching, joints	3.9
					PCC Slab replacement	3.9
					PCC Slab replacement	10.4
	DGAB				PCC Slab replacement	12.7
					Transverse joint sealing	13.3
					Lane-Shoulder, Longitudinal joint sealing	13.3
					Full Depth joint repair patching	19.4
No					Full Depth patching, not joint	19.4
					200205	
			550	12	Transverse joint sealing	13.3
			550	12	Lane-Shoulder, Longitudinal joint sealing	13.3
	LCB				Partial depth patching, joints	16.4
	LCD				Partial depth patching, joints	18.4
					Full Depth joint repair patching	19.4
					Full Depth patching, not joint	19.4
					Partial depth patching, joints	22.4
	РАТВ	8			200209	
					Transverse joint sealing	13.3
Yes					Lane-Shoulder, Longitudinal joint sealing	13.3
					Full Depth joint repair patching	19.4
					Full Depth patching, not joint	19.4
					200202	
					Transverse joint sealing	13.3
	DGAB				Lane-Shoulder, Longitudinal joint sealing	13.3
					Full Depth joint repair patching	19.4
No					Full Depth patching, not joint	19.4
					200206	
	LCB		900	14	Transverse joint sealing	13.3
	LCD				Lane-Shoulder, Longitudinal joint sealing	13.3
					PCC Slab replacement	19.4
Vac					200210	
	РАТВ				Transverse joint sealing	13.3
Yes	FAID				Lane-Shoulder, Longitudinal joint sealing	13.3
					Full Depth joint repair patching	19.4

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
					200203		
	DGAB				Transverse joint sealing	13.3	
					Lane-Shoulder, Longitudinal joint sealing	13.3	
					200207		
No					Transverse joint sealing	13.3	
	LCB				Lane-Shoulder, Longitudinal joint sealing	13.3	
	LCD		550	14	Partial depth patching, joints	16.4	
					Full Depth joint repair patching	19.4	
					Full Depth patching, not joint	19.4	
					200211		
Yes	РАТВ				Transverse joint sealing	13.3	
163	FAID				Lane-Shoulder, Longitudinal joint sealing	13.3	
		11			Full Depth joint repair patching	19.4	
					200204		
					Partial depth patching, joints	3.4	
	DGAB				Partial depth patching, joints	13.3         aling       13.3         aling       13.3         aling       13.3         16.4       19.4         19.4       19.4         aling       13.3         aling       13.3	
	DGAB				Transverse joint sealing	13.3	
No					Lane-Shoulder, Longitudinal joint sealing	13.3	
			900	12	Full Depth joint repair patching	19.4	
			900	12	200208		
	LCB				Transverse joint sealing	13.3	
					Lane-Shoulder, Longitudinal joint sealing	13.3	
					200212		
Yes	PATB				Transverse joint sealing	13.3	
					Lane-Shoulder, Longitudinal joint sealing	13.3	

Table A-8. Maintenance and Rehabilitation for si	sites in Michigan.
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Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
No	DGAB		550	14	260213	
No	LCB	8			260217	
Yes	ΡΑΤΒ				260221	
					Lane-Shoulder, Longitudinal joint sealing	9.8
					Partial depth patching, joints	15.4
No L	DGAB		900	12	260214	
					PCC Slab replacement	9.4
					Lane-Shoulder, Longitudinal joint sealing	9.4
					Full Depth joint repair patching	10.8
					Partial depth patching, joints	10.8
	LCB				260218	
	РАТВ				260222	
					Lane-Shoulder, Longitudinal joint sealing	9.8
No	DGAB		550		260215	
	LCB				260219	
					Partial depth patching, joints	10.7
					Partial depth patching, joints	13.4
Yes I	РАТВ				260223	
					Lane-Shoulder, Longitudinal joint sealing	9.8
					Partial depth patching, joints	15.4
No	DGAB	11 900		14	260216	
					Lane-Shoulder, Longitudinal joint sealing	9.8
					Partial depth patching, joints	15.4
	LCB		900		260220	
					Lane-Shoulder, Longitudinal joint sealing	9.8
				Partial depth patching, joints	15.4	
Yes	РАТВ				260224	
		Shaded cells			Lane-Shoulder, Longitudinal joint sealing	9.8

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
					320201	
					Full Depth patching, not joint	4.3
	DCAD				Crack sealing	4.7
	DGAB				Full Depth patching, not joint	6.8
No			550	12	Full Depth patching, not joint	7.3
			550	12	Full Depth patching, not joint	10.4
					320205	
	LCB				Crack sealing	2.7
		8			Crack sealing	4.7
Yes	РАТВ				320209	
					320202	
	DGAB				Crack sealing	4.3
No					Partial depth patching, not joint	4.3
					320206	
	LCB			14	Partial depth patching, not joint	4.3
Vac	DATD				320210	
Yes	ΡΑΤΒ				Crack sealing	4.7
					320203	
	DGAB	GAB			Crack sealing	2.7
N -					Crack sealing	4.7
No		LCB			320207	
	LCB		550		Partial depth patching, joints	4.3
					Crack sealing	4.7
					320211	
Yes	PATB				Crack sealing	2.7
					Crack sealing	4.7
		11			320204	
					Partial depth patching, not joint	2.7
	DCAD				Partial depth patching, not joint	3.3
	DGAB				Crack sealing	4.3
No					Partial depth patching, joints	4.3
			900	12	Crack sealing	4.7
					320208	
					Partial depth patching, not joint	2.7
	LCB				Crack sealing	4.7
					Partial depth patching, not joint	9.3
Yes	РАТВ				320212	

Table A-9. Maintenance and Rehabilitation for sites in Nevada.

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
No	DGAB				370201		
No	LCB		550	12	370205		
Yes	РАТВ				370209		
No	DGAB	8			370202		
No	LCB		000	14	370206		
Vec	РАТВ		900		370210		
Yes	PAID				Partial depth patching, joints	2.9	
No	DGAB				370203		
NO	LCB		550		370207		
Yes	РАТВ	11			370211		
No	DGAB	11			370204		
No	LCB		900	12	370208		
Yes	РАТВ				370212		

Table A-10. Maintenance and Rehabilitation for sites in North Carolina

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
					380213		
					Lane-Shoulder, Longitudinal joint sealing	6.8	
					Lane-Shoulder, Longitudinal joint sealing	11.6	
	DGAB				Transverse joint sealing	14.9	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Partial depth patching, joints	14.9	
					Grinding/Milling surface	16.5	
					380217		
					Partial depth patching, joints	3.7	
					Crack sealing	3.7	
					Partial depth patching, joints	4.8	
					Crack sealing	6.8	
No					Lane-Shoulder, Longitudinal joint sealing	6.8	
NO				14	Crack sealing	7.8	
			550		Lane-Shoulder, Longitudinal joint sealing		7.8
		8			Crack sealing	10.8	
	LCB				Lane-Shoulder, Longitudinal joint sealing	10.8	
					Partial depth patching, not joint	10.8	
					Partial depth patching, joints	10.8	
					Crack sealing	14.9	
					Transverse joint sealing	14.9	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Full Depth joint repair patching	14.9	
					PCC Slab replacement	14.9	
					Partial depth patching, joints	14.9	
					Grinding/Milling surface	16.5	
					380221	1	
					Lane-Shoulder, Longitudinal joint sealing	6.8	
Yes	PATB				Lane-Shoulder, Longitudinal joint sealing	11.6	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Grinding/Milling surface	16.5	

 Notes:
 • Shaded cells are no longer in study

 • Sections with nothing listed have not received any maintenance or rehabilitation

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age		
					380214			
					Lane-Shoulder, Longitudinal joint sealing	6.8		
					Lane-Shoulder, Longitudinal joint sealing	11.6		
	DGAB				Transverse joint sealing	14.9		
					Lane-Shoulder, Longitudinal joint sealing	14.9		
					Partial depth patching, joints	14.9		
No					Grinding/Milling surface	16.5		
NO					380218			
					Lane-Shoulder, Longitudinal joint sealing	6.8		
					Lane-Shoulder, Longitudinal joint sealing	11.6		
	LCB	11	900		Transverse joint sealing	14.9		
					Lane-Shoulder, Longitudinal joint sealing	14.9		
					Partial depth patching, joints	14.9		
					Grinding/Milling surface	16.5		
					380222	1		
				12	Lane-Shoulder, Longitudinal joint sealing	6.8		
					Lane-Shoulder, Longitudinal joint sealing	11.6		
Yes	PATB				Transverse joint sealing	14.9		
					Lane-Shoulder, Longitudinal joint sealing	14.9		
					Partial depth patching, joints	14.9		
					Grinding/Milling surface	16.5		
					380215			
					Lane-Shoulder, Longitudinal joint sealing	6.8		
					Lane-Shoulder, Longitudinal joint sealing	11.6		
	DGAB				Transverse joint sealing	14.9		
					Lane-Shoulder, Longitudinal joint sealing	14.9		
					Partial depth patching, joints	14.9		
No	No	8	550		Grinding/Milling surface	16.5		
					Skin patching	20.6		
					380219	I		
					Lane-Shoulder, Longitudinal joint sealing	6.8		
	LCB				Lane-Shoulder, Longitudinal joint sealing	11.6		
					Lane-Shoulder, Longitudinal joint sealing	14.9		
					Grinding/Milling surface	16.5		

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age	
					380223		
					Lane-Shoulder, Longitudinal joint sealing	6.8	
Yes	PATB	8	550	12	Lane-Shoulder, Longitudinal joint sealing	11.6	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Grinding/Milling surface	16.5	
					380216		
					Partial depth patching, joints	4.8	
					Partial depth patching, joints	5.8	
					Lane-Shoulder, Longitudinal joint sealing	6.8	
	DGAB			14	Lane-Shoulder, Longitudinal joint sealing	11.6	
	DOAD				Partial depth patching, joints	11.6	
					Transverse joint sealing	14.9	
No					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Partial depth patching, joints	14.9	
		11	900		Grinding/Milling surface	16.5	
		11	900	14	380220		
					Lane-Shoulder, Longitudinal joint sealing	6.8	
	LCB				Lane-Shoulder, Longitudinal joint sealing	11.6	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Grinding/Milling surface	16.5	
					380224		
					Lane-Shoulder, Longitudinal joint sealing	6.8	
Yes	PATB				Lane-Shoulder, Longitudinal joint sealing	11.6	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Grinding/Milling surface	16.5	

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
Ne	DGAB				390201	
No	LCB		550	12	390205	
Vac	DATD		550	12	390209	
Yes	PATB	8			Full Depth joint repair patching	13.5
No	DGAB				390202	
NO	LCB		900		390206	
Yes	PATB				390210	
	DGAB				390203	
	DUAD				Grinding/Milling surface	19.4
				14	390207	
No			550		Full Depth joint repair patching	18.6
	LCB		550		Full Depth patching, not joint	18.6
					PCC Slab replacement	19.4
					Other	19.4
Yes	ΡΑΤΒ				390211	
	DGAB				390204	
					390208	
	LCB				Full Depth joint repair patching	13.5
No		11			Full Depth joint repair patching	18.6
					Full Depth patching, not joint	18.6
					Full Depth patching, not joint	19.4
					PCC Slab replacement	19.4
			900	12	Other	19.4
			900	12	390212	
					Full Depth joint repair patching	13.5
					Partial depth patching, not joint	16.4
Yes	РАТВ				Partial depth patching, not joint	18.4
103					Full Depth joint repair patching	18.6
					Full Depth patching, not joint	18.6
					PCC Slab replacement	19.4
					Other	19.4

Table A-12. Maintenance and Rehabilitation for sites in Ohio

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
No	DGAB				530	201
No	LCB		550	12	530	205
Yes	РАТВ	8			530209	
No	DGAB	õ			530202	
No	LCB		900	14	530206	
Yes	РАТВ				530210	
Ne	DGAB			14	530203	
No	LCB		550		530207	
Yes	РАТВ				530211	
No	DGAB	11			530204	
INO	No LCB		900	12	530208	
Yes	РАТВ				530212	

Table A-13. Maintenance and Rehabilitation for sites in Washington

Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Code	Age
	DGAB				550213	
No	LCB		550	14	550217	
	LCB		550	14	Patch potholes, by hand	14.4
Yes	PATB	8			550221	
No	DGAB				550214	
No	LCB		900		550218	
Yes	PATB			10	550222	
No	DGAB			12	550215	
No	LCB		550		550219	
Yes	PATB	11			550223	
No	DGAB	11			550216	
No	LCB		900	14	550220	
Yes	PATB				550224	

Table A-14. Maintenance and Rehabilitation for sites in Wisconsin

Appendix B. Tables of Test Section Rehabilitation and Maintenance and Recent Distress Survey Results

					Arizona		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					0402	13	
					Partial depth patching, not joint	16.6	Pumping
					Partial depth patching, joints	16.6	Longitudinal cracking
							Transverse cracking
	DGAB						Longitudinal spalling
							Flexible patches
No						Transverse spalling	
							Scaling
							Map cracking
		LCB 8	550		040217		
				14	Partial depth patching, not joint	16.6	Transverse spalling
					Partial depth patching, joints	16.6	Longitudinal cracking
	LCB			14			Transverse cracking
							Longitudinal spalling
							Scaling
							Map cracking
					0402	21	1
					Partial depth patching, joints	14.5	Transverse spalling
					Partial depth patching, not joint	16.6	Longitudinal cracking
Yes	РАТВ				Partial depth patching, joints	16.6	Transverse cracking
							Longitudinal spalling
							Flexible patches
							Scaling
							Map cracking

Table B-1. Maintenance, Rehabilitation, and Distresses for sites in Arizona.

Notes: • Shaded cells are no longer in study

					Arizona		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					0402	214	
							Longitudinal cracking
							Transverse cracking
	DGAB						Longitudinal spalling
							Transverse spalling
							Scaling
No							Map cracking
NO					0402	218	
					Partial depth patching, joints	14.5	Transverse spalling
		8	900				Corner breaks
	LCB	0	900				Longitudinal crackin
							Transverse cracking
				12			Longitudinal spalling
							Map cracking
					0402	222	
							Longitudinal crackin
Yes	РАТВ						Transverse cracking
105	TAID						Longitudinal spalling
							Transverse spalling
				4			Map cracking
					0402	215	1
							Longitudinal crackin
No	DGAB	11	550				Longitudinal spalling
							Transverse spalling
							Map cracking

Table B-2. Maintenance, Rehabilitation, and Distresses for sites in Arizona (continued). 

Notes: • Shaded cells are no longer in study

Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
				0402	219	
						Longitudinal crackin
LCB						Transverse cracking
		550	12			Transverse spalling
		550	12			Map cracking
				0402	223	
PATB						Longitudinal spalling
						Transverse spalling
				0402	216	
DGAB	IGAB 11					Longitudinal crackin
DOND						Longitudinal spalling
						Map cracking
				0402	220	1
LCB		900	14			Longitudinal crackin
		500				Longitudinal spalling
				0402	224	1
						Map cracking
PATB						Longitudinal crackin
						Longitudinal spalling
[	PATB DGAB LCB	LCB PATB DGAB 11 LCB	LCB 550 PATB 200	LCB 550 12 PATB 11 12 LCB 900 14	LCB 550 12 040 PATB 550 12 040 DGAB 11 040 LCB 900 14 040 040 040 040 040 040 040 040	LCB 550 12 040219 PATB 040223 DGAB 11 4 040223 LCB 900 14 040220 14 040220

Table B-2. Maintenance, Rehabilitation, and Distresses for sites in Arizona (continued).

Notes: • Shaded cells are no longer in study

					Arkansas		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					050213		
					Lane-Shoulder, Longitudinal joint sealing	3.4	
	DGAB				Crack sealing	9.3	
					Partial depth patching, not joint	10.2	
					Partial depth patching, not joint	13.1	
					050217		
No					Lane-Shoulder, Longitudinal joint sealing	3.4	Corner breaks
					Crack sealing	9.3	Longitudinal cracking
	LCB		550	14	Partial depth patching, joints	9.3	Transverse cracking
	LCD		550	17	Partial depth patching, joints	12.8	Longitudinal spalling
							Flexible patches
		8					polished aggregate
							pumping
					050221		
					Lane-Shoulder, Longitudinal joint sealing	3.4	polished aggregate
Yes	PATB				Transverse joint sealing	9.3	Corner breaks
					Lane-Shoulder, Longitudinal joint sealing	9.3	Transverse cracking
							Longitudinal spalling
					050214	1	
					Lane-Shoulder, Longitudinal joint sealing	3.4	polished aggregate
No	DGAB		900	12			Transverse cracking
	20,0						Longitudinal spalling
							Transverse spalling
							pumping

# Table B-2. Maintenance, Rehabilitation, and Distresses for sites in Arkansas.

Notes: • Shaded cells are no longer in study

					Arkansas		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					050218		
					Lane-Shoulder, Longitudinal joint sealing	3.4	pumping
					Crack sealing	9.3	Corner breaks
No	LCB				Transverse joint sealing	9.3	Longitudinal cracking
					Partial depth patching, joints	12.8	Transverse cracking
		8	900				Longitudinal spalling
		0	500				Flexible patches
					050222	1	
					Lane-Shoulder, Longitudinal joint sealing	3.4	pumping
Yes	PATB						Longitudinal spalling
							Transverse spalling
							pumping
				12	050215	1	
					Lane-Shoulder, Longitudinal joint sealing	3.4	polished aggregate
	DGAB						Longitudinal spalling
							Transverse spalling
No							pumping
					050219	1	I
		11	550		Lane-Shoulder, Longitudinal joint sealing	3.4	polished aggregate
	LCB						Longitudinal spalling
							Transverse spalling
							pumping
					050223	1	
Yes	РАТВ				Lane-Shoulder, Longitudinal joint sealing	3.4	polished aggregate
		Shadad calls					Longitudinal spalling

# Table B-2. Maintenance, Rehabilitation, and Distresses for sites in Arkansas (continued).

Notes: • Shaded cells are no longer in study

					Arkansas		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					050216		
					Lane-Shoulder, Longitudinal joint sealing	3.4	polished aggregate
	DGAB				Transverse joint sealing	9.3	Longitudinal spalling
					Lane-Shoulder, Longitudinal joint sealing	9.3	Transverse spalling
							pumping
No					050220		1
					Lane-Shoulder, Longitudinal joint sealing	3.4	Transverse spalling
	LCB				Transverse joint sealing	9.3	Corner breaks
	LCD	11	900	14	Lane-Shoulder, Longitudinal joint sealing	3.4	Longitudinal spalling
							Flexible patches
							pumping
					050224	i	
					Partial depth patching, joints	2.8	polished aggregate
Yes	РАТВ				Lane-Shoulder, Longitudinal joint sealing	3.4	Longitudinal spalling
100	17.10				Partial depth patching, joints	7.8	Transverse spalling
					Transverse joint sealing	9.3	Flexible patches
							pumping

## Table B-2. Maintenance, Rehabilitation, and Distresses for sites in Arkansas (continued).

Notes: • Shaded cells are no longer in study

					California		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					060201		
					Lane-Shoulder, Longitudinal joint sealing	2.3	Transverse spalling
					Partial depth patching, not joint	8.0	Corner breaks
					Partial depth patching, not joint	13.3	Longitudinal cracking
	DGAB						Transverse cracking
							Longitudinal spalling
							Flexible patching
No							pumping
110							map cracking
					060205		
					Lane-Shoulder, Longitudinal joint sealing	2.3	Transverse spalling
		8	550	12			Longitudinal cracking
	LCB						Transverse cracking
							Longitudinal spalling
							Flexible patching
							map cracking
					060209		
					Lane-Shoulder, Longitudinal joint sealing	2.3	Transverse spalling
					Partial depth patching, joints	13.3	Scaling
Yes	PATB						Longitudinal cracking
							Transverse cracking
							Longitudinal spalling
							map cracking

## Table B-3. Maintenance, Rehabilitation, and Distresses for sites in California.

Notes: • Shaded cells are no longer in study

					California		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					060202		
					Lane-Shoulder, Longitudinal joint sealing	5.3	Longitudinal cracking
	DGAB						Transverse cracking
							Longitudinal spalling
							map cracking
No					060206	1	
NO					Transverse joint sealing	8.0	Transverse spalling
					Lane-Shoulder, Longitudinal joint sealing	8.0	Corner breaks
	LCB	8	900				Longitudinal cracking
							Transverse cracking
							Longitudinal spalling
				14			map cracking
				14	060210	I	1
					Lane-Shoulder, Longitudinal joint sealing	5.3	Longitudinal spalling
Yes	PATB				Transverse joint sealing	8.0	map cracking
					Lane-Shoulder, Longitudinal joint sealing	8.0	
					060203		
					Transverse joint sealing	2.3	Transverse spalling
					Lane-Shoulder, Longitudinal joint sealing	2.3	Longitudinal cracking
No	DGAB	11	550		Grinding/Milling surface	5.3	Transverse cracking
					Grinding/Milling surface	9.0	Longitudinal spalling
							Scaling
							map cracking

## Table B-3. Maintenance, Rehabilitation, and Distresses for sites in California (continued).

Notes: • Shaded cells are no longer in study

					California	-	
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					060207		
					Transverse joint sealing	8.0	Transverse spalling
					Lane-Shoulder, Longitudinal joint sealing	8.0	Longitudinal cracking
No	LCB						Transverse cracking
NO	LCD						Longitudinal spalling
							Scaling
			550	14			polished aggregate
							map cracking
					060211		
					Transverse joint sealing	2.3	Transverse spalling
Yes	ΡΑΤΒ				Lane-Shoulder, Longitudinal joint sealing	2.3	Longitudinal spalling
		11					Scaling
		11					map cracking
					060204		
					Lane-Shoulder, Longitudinal joint sealing	5.3	Transverse spalling
	DGAB				Transverse joint sealing	8.0	Transverse cracking
					Lane-Shoulder, Longitudinal joint sealing	8.0	Longitudinal spalling
							map cracking
No			900	12	060208		
					Lane-Shoulder, Longitudinal joint sealing	2.3	Transverse spalling
	LCB				Lane-Shoulder, Longitudinal joint sealing	5.3	Longitudinal cracking
	100				Lane-Shoulder, Longitudinal joint sealing	8.0	Transverse cracking
							Longitudinal spalling
							map cracking

### Table B-3. Maintenance, Rehabilitation, and Distresses for sites in California (continued).

Notes: • Shaded cells are no longer in study

#### Table B-3. Maintenance, Rehabilitation, and Distresses for sites in California (continued).

					California		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					060212		
Yes	ΡΑΤΒ	11	900	12	Lane-Shoulder, Longitudinal joint sealing	5.3	Corner breaks Longitudinal spalling map cracking

Notes: • Shaded cells are no longer in study

					Colorado		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					C	080213	1
							Longitudinal cracking
							Longitudinal spalling
	DGAB						transverse spalling
							scaling
No							polished aggregate
							map cracking
					C	080217	
	LCB		550	14	Partial depth patching, joints	12.4	
					Partial depth patching, not joint	17.5	
		8			C	080221	
							Longitudinal cracking
							Longitudinal spalling
Yes	PATB						transverse spalling
							scaling
							polished aggregate
							map cracking
					C	080214	1
							Longitudinal cracking
No	DGAB		900	12			Transverse cracking
							Longitudinal spalling
							transverse spalling

#### Table B-4. Maintenance, Rehabilitation, and Distresses for sites in Colorado.

Notes: • Shaded cells are no longer in study

					Colorado		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					08021	8	
					Partial depth patching, joints	12.4	polished aggregate
					Partial depth patching, joints	15.4	Durability cracking
							Longitudinal cracking
No	LCB						Transverse cracking
NO	LCD						Longitudinal spalling
							Rigid patching
		8	900				Flexible patching
		0	500				transverse spalling
							map cracking
					08022	2	1
					Partial depth patching, joints	6.4	Longitudinal cracking
Yes	РАТВ				Partial depth patching, joints	11.4	Longitudinal spalling
				12	Partial depth patching, joints	12.4	transverse spalling
							map cracking
				-	08021	5	
					Partial depth patching, joints	11.4	transverse spalling
					Partial depth patching, joints	13.4	Corner breaks
	DGAB						Longitudinal spalling
							Flexible patching
No		11	550				polished aggregate
							map cracking
					08021	9	
	LCB						Longitudinal cracking
	LCD						scaling
							polished aggregate

## Table B-4. Maintenance, Rehabilitation, and Distresses for sites in Colorado (continued).

• Sections with nothing listed have not received any maintenance or rehabilitation

					Colorado		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					080223		
							map cracking
							Transverse cracking
							Longitudinal spalling
Yes	PATB		550	12			transverse spalling
							scaling
							polished aggregate
							map cracking
							transverse spalling
					080216	1	
		11			Partial depth patching, not joint	12.4	Longitudinal cracking
							Transverse cracking
	DGAB						Longitudinal spalling
							Flexible patching
No			900	14			transverse spalling
							map cracking
					080220	)	
							Flexible patching
	LCB						polished aggregate
							transverse spalling
							map cracking

Table B-4. Maintenance, Rehabilitation, and Distresses for sites in Colorado (continued).

Notes: • Shaded cells are no longer in study

#### Table B-4. Maintenance, Rehabilitation, and Distresses for sites in Colorado (continued).

					Colorado		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					80224		
					Partial depth patching, joints	11.3	Longitudinal spalling
							Longitudinal cracking
							Flexible patching
No	PATB	11	900	14			transverse spalling
							scaling
							polished aggregate
							map cracking

Notes: • Shaded cells are no longer in study

					Delaware											
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress									
					100201											
					Full Depth joint repair patching	4.3	Transverse spalling									
	DGAB				Grinding/Milling surface	8.4	Longitudinal spalling									
	00/10	40					Rigid patching									
							polished aggregate									
							map cracking									
No					100205	i	I									
					Full Depth joint repair patching	4.3	Longitudinal cracking									
	LCB				PCC Slab replacement	4.3	Transverse cracking									
		550	12	Grinding/Milling surface	8.4	Longitudinal spalling										
					Transverse joint sealing	15.4	Rigid patching									
					8	8	8	8	8	8	8			Lane-Shoulder, Longitudinal joint sealing	15.4	Transverse spalling
		Ŭ	U U	0										Full Depth joint repair patching	20.2	polished aggregate
					Skin patching	22.4	map cracking									
					100209	1	I									
					Full Depth joint repair patching	4.3	Transverse spalling									
Yes	PATB				Grinding/Milling surface	8.4	Longitudinal spalling									
							Rigid patching									
							polished aggregate									
					100202		I									
							Longitudinal cracking									
No	DGAB		900	14			Longitudinal spalling									
							Transverse spalling									
							polished aggregate									

## Table B-5. Maintenance, Rehabilitation, and Distresses for sites in Delaware.

Notes: • Shaded cells are no longer in study

					Delaware		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					100206	5	
No	LCB						Transverse spalling
NO	LCD						polished aggregate
							map cracking
					100210	)	1
					Full Depth joint repair patching	4.3	scaling
		8	900		Grinding/Milling surface	8.4	Longitudinal cracking
					Transverse joint sealing	10.4	Longitudinal spalling
Yes	PATB				Lane-Shoulder, Longitudinal joint sealing	10.4	Rigid patching
					Skin patching	22.4	Flexible patching
							Transverse spalling
							polished aggregate
				14			map cracking
				17	100203	3	l
					Skin patching	22.4	Transverse spalling
	DGAB						Longitudinal cracking
	00,10						Longitudinal spalling
							Flexible patching
							map cracking
No		11	550		100207	7	I
					Lane-Shoulder, Longitudinal joint sealing	14.4	Longitudinal cracking
					Crack sealing	16.4	Sealed longitudinal cracks
	LCB				Other	16.4	Longitudinal spalling
					Crack sealing	19.2	Flexible patching
					Partial depth patching, not joint	19.2	map cracking
				Patch potholes, by hand	19.2	Transverse spalling	

## Table B-5. Maintenance, Rehabilitation, and Distresses for sites in Delaware (continued).

			Skin patching	22.4	polished aggregate	
•••		 				

• Sections with nothing listed have not received any maintenance or rehabilitation

#### Table B-5. Maintenance, Rehabilitation, and Distresses for sites in Delaware (continued).

					Delaware				
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress		
					100211				
					Lane-Shoulder, Longitudinal joint sealing	14.4	Longitudinal spalling		
Yes	PATB		550	14	Partial depth patching, not joint	19.2	Flexible patching		
					Skin patching	22.4	polished aggregate		
							map cracking		
					100204	1	1		
D					Grinding/Milling surface	8.4	Longitudinal spalling		
	DGAB	БАВ			Partial depth patching, joints	19.4	Flexible patching		
					Skin patching 22.4 Transverse		Transverse spalling		
		11					map cracking		
					100208	3	1		
					Grinding/Milling surface	8.4	polished aggregate		
No					Lane-Shoulder, Longitudinal joint sealing	14.4	Corner breaks		
			900	12	Patch potholes, by hand	19.2	Longitudinal spalling		
	LCB		500		Partial depth patching, not joint	20.3	Rigid patching		
	200				Partial depth patching, joints	20.3	Flexible patching		
					Skin patching	22.4	Transverse spalling		
							scaling		
							pumping		
							map cracking		
					100212	2	I		
Yes	PATB				Grinding/Milling surface	8.4	Longitudinal spalling		
					Skin patching	22.4	Flexible patching		

			map cracking

					lowa				
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress		
	DGAB				190213				
					190217				
					PCC Slab replacement	11.4	Longitudinal cracking		
					Transverse joint sealing	12.4	Longitudinal spalling		
No					Lane-Shoulder, Longitudinal joint sealing	12.4	Rigid patching		
NO	LCB		550	14	Full Depth patching, not joint	12.4	Flexible patching		
			550	14	PCC Slab replacement	12.4			
					PCC Slab replacement	19.4			
					Partial depth patching, joints	19.4			
					Skin patching	20.4			
Yes	РАТВ				190221				
		8			Longitudinal spalling				
					190214	i			
	DGAB				Crack sealing	19.4	Transverse spalling		
	00/10						Transverse cracking		
							Longitudinal spalling		
					190218	1			
No			900	12	Partial depth patching, not joint	19.4	Transverse spalling		
					Partial depth patching, joints	19.4	Corner break		
	LCB				Skin patching	20.4	Longitudinal cracking		
							Transverse cracking		
							Longitudinal spalling		
							Flexible patching		

## Table B-6. Maintenance, Rehabilitation, and Distresses for sites in Iowa.

Notes: • Shaded cells are no longer in study

					Iowa				
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress		
					190222	22			
Yes	РАТВ	8	900		Partial depth patching, joints	19.4	Transverse spalling		
res	PAID	0	900		Skin patching	20.4	Longitudinal spalling		
							Flexible patching		
					190215				
	DGAB						Longitudinal spalling		
				12			Transverse spalling		
No					190219				
	LCB		550	550				Longitudinal spalling	
	LCD							polished aggregate	
							Transverse spalling		
Yes	РАТВ				190223		1		
105							Transverse spalling		
							190216		1
		11			Skin patching	20.4	Longitudinal spalling		
	DGAB						Transverse spalling		
	00,10						Longitudinal cracking		
No							Transverse cracking		
							Flexible patching		
			900	14	190220	i	I		
	LCB				Partial depth patching, joints	19.4	Transverse spalling		
					Skin patching	20.4	Longitudinal spalling		
					190224	1	I		
Yes	РАТВ				Partial depth patching, joints	19.4	Longitudinal spalling		
					Partial depth patching, not joint	19.4	Flexible patching		
					Skin patching	20.4	Transverse spalling		

### Table B-6. Maintenance, Rehabilitation, and Distresses for sites in Iowa (continued).

Notes: • Shaded cells are no longer in study

					Kansas					
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress			
					200201					
					Partial depth patching, joints	3.9	Longitudinal cracking			
					PCC Slab replacement	3.9	Transverse cracking			
					PCC Slab replacement	10.4	Longitudinal spalling			
	DGAB				PCC Slab replacement	12.7	Rigid patching			
					Transverse joint sealing	13.3	Transverse spalling			
					Lane-Shoulder, Longitudinal joint sealing	13.3				
					Full Depth joint repair patching	19.4				
No		_			Full Depth patching, not joint	19.4				
					200205	1	1			
		8	550	12	Transverse joint sealing	13.3	Longitudinal spalling			
		-			Lane-Shoulder, Longitudinal joint sealing	13.3	Rigid patching			
	LCB				Partial depth patching, joints	16.4	Flexible patching			
					Partial depth patching, joints	18.4	Transverse spalling			
					Full Depth joint repair patching	19.4	map cracking			
					Full Depth patching, not joint	19.4				
		-			Partial depth patching, joints	22.4				
					200209	1	l			
					Transverse joint sealing	13.3	Longitudinal cracking			
Yes	ΡΑΤΒ				Lane-Shoulder, Longitudinal joint sealing	13.3	Longitudinal spalling			
					Full Depth joint repair patching	19.4	Rigid patching			
					Full Depth patching, not joint	19.4				

#### Table B-7. Maintenance, Rehabilitation, and Distresses for sites in Kansas.

Notes: • Shaded cells are no longer in study

					Kansas																																
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress																														
					200202																																
					Transverse joint sealing	13.3	Transverse spalling																														
	DGAB				Lane-Shoulder, Longitudinal joint sealing	13.3	Longitudinal cracking																														
	DGAB			Full Depth joint repair patching	19.4	Transverse cracking																															
					Full Depth patching, not joint	19.4	Longitudinal spalling																														
No							Rigid patching																														
					200206	1																															
		8	900		Transverse joint sealing	13.3	Transverse spalling																														
	LCB	C	900	500		Lane-Shoulder, Longitudinal joint sealing	13.3	Longitudinal cracking																													
					PCC Slab replacement	19.4	Rigid patching																														
							map cracking																														
																																			200210	i	
Yes	ΡΑΤΒ																									Lane-Shoulder, Longitudinal joint sealing	13.3	Longitudinal cracking									
					Full Depth joint repair patching	19.4	Longitudinal spalling																														
							Rigid patching																														
					200203	I																															
	DGAB				Transverse joint sealing	13.3	Longitudinal cracking																														
					Lane-Shoulder, Longitudinal joint sealing	13.3																															
					200207	1																															
No		11	550		Transverse joint sealing	13.3	Longitudinal spalling																														
	LCB				Lane-Shoulder, Longitudinal joint sealing	13.3	Rigid patching																														
					Partial depth patching, joints	16.4	Transverse spalling																														
					Full Depth joint repair patching	19.4	map cracking																														
					Full Depth patching, not joint	19.4																															

## Table B-7. Maintenance, Rehabilitation, and Distresses for sites in Kansas.

Notes: • Shaded cells are no longer in study

					Kansas		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					200211		
					Transverse joint sealing	13.3	Transverse spalling
Yes	РАТВ		550	14	Lane-Shoulder, Longitudinal joint sealing	13.3	Longitudinal cracking
105	IAID		550	14	Full Depth joint repair patching	19.4	Longitudinal spalling
							Rigid patching
							map cracking
					200204	1	1
					Partial depth patching, joints	3.4	Longitudinal spalling
	DGAB				Partial depth patching, joints	5.3	Rigid patching
	20,12	11			Transverse joint sealing	13.3	Transverse spalling
No					Lane-Shoulder, Longitudinal joint sealing	13.3	map cracking
					Full Depth joint repair patching	19.4	
			900	12	200208		
	LCB				Transverse joint sealing	13.3	Longitudinal spalling
					Lane-Shoulder, Longitudinal joint sealing	13.3	map cracking
					200212		
Yes	РАТВ				Transverse joint sealing	13.3	Transverse spalling
163	1710				Lane-Shoulder, Longitudinal joint sealing	13.3	Longitudinal cracking
							Longitudinal spalling

#### Table B-7. Maintenance, Rehabilitation, and Distresses for sites in Kansas.

Notes: • Shaded cells are no longer in study

					Michigan												
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress										
Ne	DGAB				260213												
No	LCB				260217												
			550	14	260221												
Yes	РАТВ				Lane-Shoulder, Longitudinal joint sealing	9.8											
						Partial depth patching, joints	15.4										
					260214												
		8			PCC Slab replacement	9.4											
No	DGAB	DGAB			Lane-Shoulder, Longitudinal joint sealing	9.4											
NO			900		Full Depth joint repair patching	10.8											
			500		Partial depth patching, joints	10.8											
	LCB						1								260218		
Yes	РАТВ							260222									
105	17(10			12	Lane-Shoulder, Longitudinal joint sealing	9.8											
	DGAB	-			260215												
No					260219												
110	LCB				Partial depth patching, joints	10.7											
		11	550		Partial depth patching, joints	13.4											
					260223	-											
Yes	ΡΑΤΒ				Lane-Shoulder, Longitudinal joint sealing	9.8											
					Partial depth patching, joints	15.4											

Table B-8. Maintenance, Rehabilitation, and Distresses for sites in Michigan.

Notes: • Shaded cells are no longer in study

					Michigan		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
No	DGAB	11	900	14	260216		
					Lane-Shoulder, Longitudinal joint sealing	9.8	
					Partial depth patching, joints	15.4	
	LCB				260220		
					Lane-Shoulder, Longitudinal joint sealing	9.8	
					Partial depth patching, joints	15.4	
Yes	РАТВ				260224		
					Lane-Shoulder, Longitudinal joint sealing	9.8	

					Nevada				
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity Ag		2014/2015 Distress		
					320201				
					Full Depth patching, not joint	4.3			
	DGAB				Crack sealing	4.7			
	00,0				Full Depth patching, not joint	6.8			
No			550	12	Full Depth patching, not joint	7.3			
					Full Depth patching, not joint	10.4			
					320205	1			
	LCB				Crack sealing	2.7			
		8			Crack sealing	4.7			
Yes	PATB				320209				
						320202	1		
	DGAB				Crack sealing	4.3			
No					000		Partial depth patching, not joint	4.3	
	LCB		900		320206				
					Partial depth patching, not joint 320210	4.3			
Yes	PATB								
					Crack sealing 320203	4.7			
	DGAB			14					
	DUAD				Crack sealing Crack sealing	2.7 4.7			
No					320207	4.7			
	LCB	11	550		Partial depth patching, joints	4.3			
					Crack sealing	4.7			
					320211	,			
Yes	PATB				Crack sealing	2.7			
					Crack sealing	4.7			

Table B-9. Maintenance, Rehabilitation, and Distresses for sites in Nevada.

Notes: • Shaded cells are no longer in study

_					Nevada		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					320204		
					Partial depth patching, not joint	2.7	
	DGAB				Partial depth patching, not joint	3.3	
	DOVD				Crack sealing	4.3	
No					Partial depth patching, joints	4.3	
NO		11	900	12	Crack sealing	4.7	
					320208		
	LCB				Partial depth patching, not joint	2.7	
	LCD				Crack sealing	4.7	
					Partial depth patching, not joint	9.3	
Yes	ΡΑΤΒ				320212		

Table B-9. Maintenance, Rehabilitation, and Distresses for sites in Nevada (continued).

Notes: • Shaded cells are no longer in study

					North Carolina		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
No	DGAB				370201		
NO	LCB		550	12	370205		
Yes	ΡΑΤΒ				370209		
No	DGAB	8			370202		
NO	LCB		900		370206		
Yes	РАТВ		500		370210	1	
100					Partial depth patching, joints	2.9	
					370203		1
							Longitudinal spalling
	DGAB						Transverse spalling
							map cracking
No							polished aggregate
				14	370207		1
							Longitudinal spalling
	LCB	11	550				Transverse spalling
							polished aggregate
							map cracking
					370211		1
							Longitudinal spalling
Yes	PATB						map cracking
							scaling
							polished aggregate

Table B-10. Maintenance, Rehabilitation, and Distresses for sites in North Carolina.

Notes: • Shaded cells are no longer in study

#### Table B-10. Maintenance, Rehabilitation, and Distresses for sites in North Carolina (continued).

					North Carolina		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					370204		
	DGAB						Longitudinal spalling
							scaling
No					370208		
	LCB						Longitudinal spalling
	LCD	11	900	12			scaling
							Transverse spalling
					370212		
Yes	РАТВ						Longitudinal spalling
165	FAID						Flexible patching
							Transverse spalling

Notes: • Shaded cells are no longer in study

					North Dakota		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					380213		
					Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal spalling
					Lane-Shoulder, Longitudinal joint sealing	11.6	Rigid patching
	DGAB				Transverse joint sealing	14.9	Transverse spalling
					Lane-Shoulder, Longitudinal joint sealing	14.9	polished aggregate
					Partial depth patching, joints	14.9	
					Grinding/Milling surface	16.5	
					380217	1	
					Partial depth patching, joints	3.7	Longitudinal cracking
					Crack sealing	3.7	Sealed longitudinal cracks
					Partial depth patching, joints	4.8	Transverse cracking
					Crack sealing	6.8	Longitudinal spalling
No		8	550	14	Lane-Shoulder, Longitudinal joint sealing	6.8	Rigid patching
		Ū	330		Crack sealing	7.8	polished aggregate
					Lane-Shoulder, Longitudinal joint sealing	7.8	
					Crack sealing	10.8	
	LCB				Lane-Shoulder, Longitudinal joint sealing	10.8	
					Partial depth patching, not joint	10.8	
					Partial depth patching, joints	10.8	
					Crack sealing	14.9	
					Transverse joint sealing	14.9	
					Lane-Shoulder, Longitudinal joint sealing	14.9	
					Full Depth joint repair patching	14.9	
					PCC Slab replacement	14.9	
					Partial depth patching, joints	14.9	
					Grinding/Milling surface	16.5	

#### Table B-11. Maintenance, Rehabilitation, and Distresses for sites in North Dakota.

Notes: • Shaded cells are no longer in study

					North Dakota		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					380221		
					Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal spalling
Yes	PATB		550	14	Lane-Shoulder, Longitudinal joint sealing	11.6	Rigid patching
					Lane-Shoulder, Longitudinal joint sealing	14.9	Transverse spalling
					Grinding/Milling surface	16.5	polished aggregate
					380214	1	1
					Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal cracking
					Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal spalling
	DGAB				Transverse joint sealing	14.9	Rigid patching
					Lane-Shoulder, Longitudinal joint sealing	14.9	Transverse spalling
					Partial depth patching, joints	14.9	polished aggregate
No					Grinding/Milling surface	16.5	
		8			380218	I	1
		•			Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal cracking
					Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal spalling
	LCB		900	12	Transverse joint sealing	14.9	Rigid patching
					Lane-Shoulder, Longitudinal joint sealing	14.9	Flexible patching
					Partial depth patching, joints	14.9	Transverse spalling
					Grinding/Milling surface	16.5	polished aggregate
					380222	1	1
					Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal cracking
					Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal spalling
Yes	PATB				Transverse joint sealing	14.9	Rigid patching
					Lane-Shoulder, Longitudinal joint sealing	14.9	Transverse spalling
					Partial depth patching, joints	14.9	
					Grinding/Milling surface	16.5	

# Table B-11. Maintenance, Rehabilitation, and Distresses for sites in North Dakota (continued).

Notes: • Shaded cells are no longer in study

					North Dakota		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					380215		
					Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal cracking
					Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal spalling
	DGAB				Transverse joint sealing	14.9	Rigid patching
	DOAD				Lane-Shoulder, Longitudinal joint sealing	14.9	Flexible patching
					Partial depth patching, joints	14.9	Transverse spalling
					Grinding/Milling surface	16.5	polished aggregate
No		-			Skin patching	20.6	
					380219		
		11	550	12	Lane-Shoulder, Longitudinal joint sealing	6.8	Transverse spalling
			550	12	Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal cracking
	LCB				Lane-Shoulder, Longitudinal joint sealing	14.9	Longitudinal spalling
					Grinding/Milling surface	16.5	Rigid patching
							Flexible patching
		-					polished aggregate
					380223		1
					Lane-Shoulder, Longitudinal joint sealing	6.8	Longitudinal spalling
Yes	PATB				Lane-Shoulder, Longitudinal joint sealing	11.6	Transverse spalling
					Lane-Shoulder, Longitudinal joint sealing	14.9	polished aggregate
					Grinding/Milling surface	16.5	

### Table B-11. Maintenance, Rehabilitation, and Distresses for sites in North Dakota (continued).

Notes: • Shaded cells are no longer in study

					North Dakota		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
					380216		
					Partial depth patching, joints	4.8	Longitudinal spalling
					Partial depth patching, joints	5.8	Rigid patching
					Lane-Shoulder, Longitudinal joint sealing	6.8	Transverse spalling
	DGAB				Lane-Shoulder, Longitudinal joint sealing	11.6	
	DOAD				Partial depth patching, joints	11.6	
					Transverse joint sealing	14.9	
No					Lane-Shoulder, Longitudinal joint sealing	14.9	
110					Partial depth patching, joints	14.9	
		-			Grinding/Milling surface	16.5	
		11	900	14	380220	I	
					Lane-Shoulder, Longitudinal joint sealing	6.8	Transverse spalling
	LCB				Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal cracking
	LCD				Lane-Shoulder, Longitudinal joint sealing	14.9	Transverse cracking
					Grinding/Milling surface	16.5	Longitudinal spalling
		-					polished aggregate
					380224	I	
					Lane-Shoulder, Longitudinal joint sealing	6.8	Transverse spalling
Yes	PATB				Lane-Shoulder, Longitudinal joint sealing	11.6	Longitudinal spalling
					Lane-Shoulder, Longitudinal joint sealing	14.9	Rigid patching
					Grinding/Milling surface	16.5	Flexible patching

# Table B-11. Maintenance, Rehabilitation, and Distresses for sites in North Dakota (continued).

Notes: • Shaded cells are no longer in study

					Ohio			
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity		Age	2014/2015 Distress
No	DGAB					390201		
NO	LCB		550	12		390205		
Yes	PATB		550	12		390209		
165		8			Full Depth joint repair patching		13.5	
No	DGAB					390202		
NO	LCB		900			390206		
Yes	ΡΑΤΒ					390210		
						390203		
	DGAB				Grinding/Milling surface		19.4	Transverse spalling
	DOAD							Longitudinal spalling
								Transverse cracking
No				14		390207		
				14	Full Depth joint repair patching		18.6	Transverse cracking
	LCB	11	550		Full Depth patching, not joint		18.6	Longitudinal spalling
					PCC Slab replacement		19.4	Rigid patching
					Other		19.4	Transverse spalling
						390211		
Yes	РАТВ							Transverse cracking
105								Longitudinal spalling
								Transverse spalling

Table B-12. Maintenance, Rehabilitation, and Distresses for sites in Ohio.

Notes: • Shaded cells are no longer in study

					Ohio		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
	DGAB				390204		
					390208		
					Full Depth joint repair patching	13.5	Longitudinal cracking
No					Full Depth joint repair patching	18.6	Transverse cracking
	LCB				Full Depth patching, not joint	18.6	Longitudinal spalling
					Full Depth patching, not joint	19.4	Rigid patching
					PCC Slab replacement	19.4	Transverse spalling
		11	900	12	Other	19.4	map cracking
		11	500	12	390212		
					Full Depth joint repair patching	13.5	Corner break
					Partial depth patching, not joint	16.4	Longitudinal cracking
Yes	РАТВ				Partial depth patching, not joint	18.4	Transverse cracking
103	IAID				Full Depth joint repair patching	18.6	Longitudinal spalling
					Full Depth patching, not joint	18.6	Rigid patching
					PCC Slab replacement	19.4	Transverse spalling
					Other	19.4	map cracking

# Table B-12. Maintenance, Rehabilitation, and Distresses for sites in Ohio (continued).

Notes: • Shaded cells are no longer in study

					Washington		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
						53	0201
							Corner break
							Longitudinal cracking
	DGAB						Sealed longitudinal cracks
							Transverse cracking
							Sealed transverse cracks
No							map cracking
						53	0205
							Corner break
		8	550	12			Sealed longitudinal cracks
	LCB						Transverse cracking
							Sealed transverse cracks
							Transverse spalling
							map cracking
						53	0209
							Corner break
Yes	PATB						Sealed longitudinal cracks
							Transverse cracking
							map cracking

 Table B-13. Maintenance, Rehabilitation, and Distresses for sites in Washington.

Notes: • Shaded cells are no longer in study

					Washington	1	
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
						53	0202
							Corner break
	DGAB						Longitudinal cracking
	DOAD						Durability cracking
							Sealed longitudinal cracks
							Transverse cracking
						53	0206
							Corner break
No							Longitudinal cracking
							Durability cracking
							Sealed longitudinal cracks
	LCB	8	900	14			Transverse cracking
							Sealed transverse cracks
							Longitudinal spalling
							Transverse spalling
							Pumping
							map cracking
						53	0210
							Corner break
Yes	РАТВ						Durability cracking
							Longitudinal cracking
							Sealed longitudinal cracks
							Transverse cracking

Table B-13. Maintenance, Rehabilitation, and Distresses for sites in Washington (continued).

Notes: • Shaded cells are no longer in study

					Washington	l	
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
	DGAB	11	550	14	530203		
							Corner break
							Sealed longitudinal cracks
							Transverse cracking
							map cracking
	LCB				530207		
No							Corner break
							Durability cracking
							Sealed longitudinal cracks
							Transverse cracking
							Sealed transverse cracks
							Transverse spalling
							map cracking
Yes	РАТВ				530211		
							Corner break
. 05							Sealed longitudinal cracks
							Transverse cracking

# Table B-13. Maintenance, Rehabilitation, and Distresses for sites in Washington (continued).

Notes: • Shaded cells are no longer in study

					Washington	l		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress	
					530204			
	DGAB		900	12			Corner break	
							Sealed longitudinal cracks	
							Transverse cracking	
							Rigid patching	
No	LCB	11			530208			
							Corner break	
							Longitudinal cracking	
							Durability cracking	
							Sealed longitudinal cracks	
							Transverse cracking	
							map cracking	
	РАТВ					53	0212	
							Corner break	
Yes							Longitudinal cracking	
							Durability cracking	
							Sealed longitudinal cracks	
							Transverse cracking	
							map cracking	

# Table B-13. Maintenance, Rehabilitation, and Distresses for sites in Washington (continued).

Notes: • Shaded cells are no longer in study

					Wisconsin		
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress
			550	14		550213	
	DGAB	-					Longitudinal spalling
							Flexible patching
No							Transverse spalling
						550217	
	LCB				Patch potholes, by hand	14.4	Longitudinal spalling
		-					Transverse spalling
	РАТВ					550221	1
Yes		РАТВ					Longitudinal spalling
							Transverse spalling
	DGAB LCB	8	900	12		550214	1
		DGAB					Longitudinal spalling
							Transverse spalling
No						550218	1
							Longitudinal cracking
							Longitudinal spalling
							Transverse spalling
Yes	РАТВ	РАТВ				550222	1
							Longitudinal cracking
							Longitudinal spalling
							Transverse spalling
							scaling

 Table B-14. Maintenance, Rehabilitation, and Distresses for sites in Wisconsin.

Notes: • Shaded cells are no longer in study

#### Table B-14. Maintenance, Rehabilitation, and Distresses for sites in Wisconsin (continued).

		Wisconsin								
Drainage	Base Type	PCC Thickness, in	Flexural strength, 14-d (psi)	Lane width	Maintenance Activity	Age	2014/2015 Distress			
						550215				
	DGAB						Longitudinal spalling			
No							Transverse spalling			
NO	LCB					550219				
			550	12			Longitudinal spalling			
							Transverse spalling			
	РАТВ							550223	1	
Yes							Longitudinal spalling			
				11					Transverse spalling	
	DGAB					550216	1			
		DGAB	DGAB	DGAB						Longitudinal spalling
No							Transverse spalling			
	LCB	LCB	LCB		900	14		550220	1	
			500				Longitudinal spalling			
	РАТВ	РАТВ				550224	1			
Yes							Longitudinal spalling			
							Transverse spalling			

Notes: • Shaded cells are no longer in study